

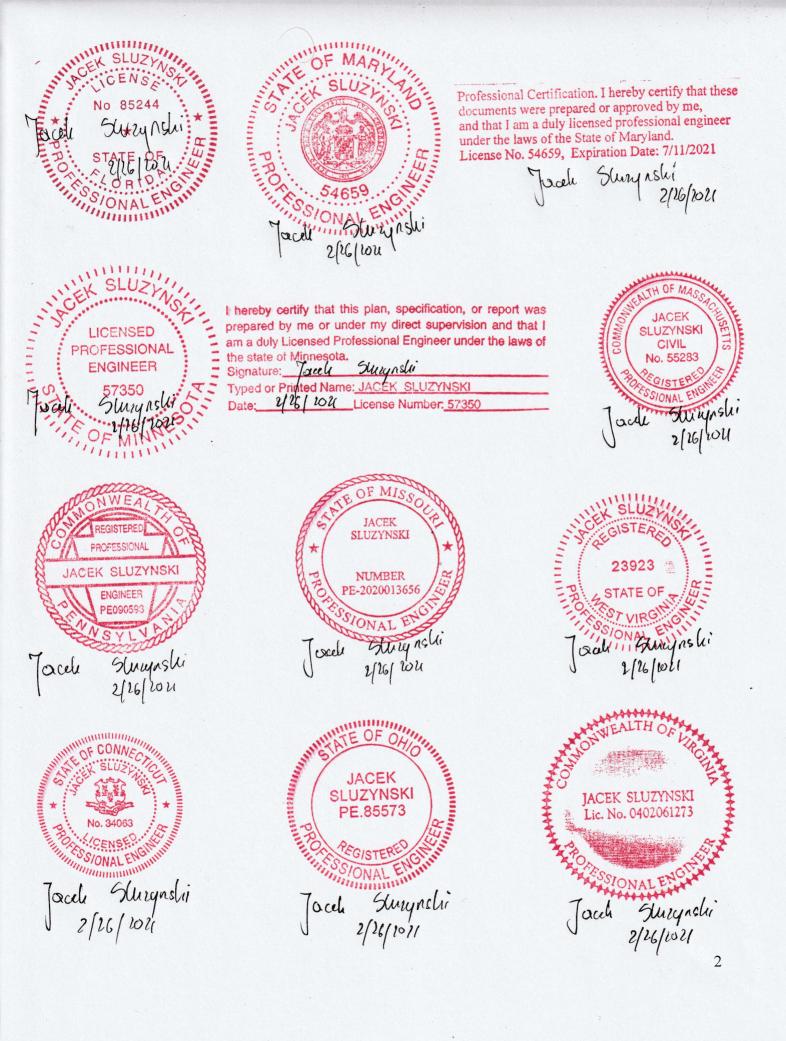
ENGINEERING REPORT PREPARED FOR

Barrette Outdoor Living

BOL ALUMINUM RAILING SYSTEM-LOAD ANALYSIS IBC 2018 AND IRC 2018

2/26/2021

JACEK SLUZYNSKI, P.E. BARRETTE OUTDOOR LIVING 2401 CORPORATE BLVD • BROOKSVILLE, FL 34604 (352) 754-8555



BOL Aluminum Railing System Load Analysis

<u>Scope</u>

The Barrette Outdoor Living Aluminum Railing System is a powder-coated aluminum railing system. Because of Aluminum construction, it is considered to be a traditional building product. According to the 2018 International Building Code, this type of product need only be proven to comply with regulations by way of written calculations. The following document will describe the system performance requirements outlined in the IBC and IRC and show the appropriate calculations needed for compliance. This analysis is also valid for the 2009, 2012 & 2015 IBC/ IRC. The Barrette Aluminum Railing system will be evaluated at 69", 93", and 96" lengths.

Sections

- I. Key Terms and Definitions
- II. Geometric Properties
- III. Structural Post Load Requirements
- IV. Top Rail Load Requirements
- V. Infill
- VI. Fastener Considerations
- VII. Conclusion

I. Key Terms and Definitions

Elastic Section Modulus (S): A geometric property for a given cross-section used in the design of beams or flexural members. The elastic section modulus is defined as S = I / y, where I is the second moment of area and y is the distance from the neutral axis to any given fiber.

Modulus of Elasticity (E): The elastic modulus of an object is defined as the slope of its stress–strain curve in the elastic deformation region.

Yield Strength (\sigma): The stress at which a predetermined amount of permanent deformation occurs.

Second Moment of Inertia (I): A property of a cross-section that can be used to predict the resistance of a beam to bending and deflection around an axis that lies in the cross-sectional plane.

Factor of Safety (FOS): Term describing the structural capacity of a system beyond the expected loads or actual loads.

II. <u>Geometric Properties</u>

The aluminum rail and post extrusions in the Avalon Railing system are created from aluminum alloy AI-6005-T5. The square baluster is created from AI-6063-T5 and the twisted baluster is created from AI-6063-T6.

The following calculations will be based on the material properties listed below. The specific numbers used in this analysis have been taken from a reputable material properties website (www.matweb.com).¹

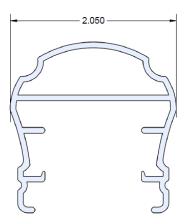
Al-6005-T5: $\sigma_{yr} = 34,800 \, psi$ Al-6063-T5: $\sigma_{yr} = 21,000 \, psi$ Al-6063-T6: $\sigma_{yr} = 31,000 \, psi$

 $\Omega_b = 1.67 (AISC F1)$

Due to the complex shape of the support extrusion used in the top rail of the Barrette Aluminum Railing system, the mass properties command in SolidWorks was used to calculate the geometric properties for each profile.

1. Top Rails

a. Cambridge Top Rail:



Section properties of the selected face of EPN 0565

Area = 0.617 inches^2

Centroid relative to output coordinate system origin: (inches) $\begin{array}{l} X=0.000\\ Y=0.126\\ Z=69.000 \end{array}$

Moments of inertia of the area, at the centroid: (inches ^ 4)

¹ Date from Material Property Sheet (Appendix)

Lxx = 0.218	Lxy = 0.000	Lxz = 0.000
Lyx = 0.000	Lyy = 0.330	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 0.548

Polar moment of inertia of the area, at the centroid = 0.548 inches ^ 4

Angle between principal axes and part axes = 0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4)

lx = 0.218 ly = 0.330

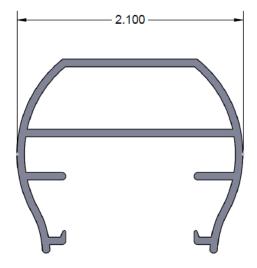
Moments of inertia of the area, at the outpu	t coordinate system:	(inches ^ 4)
LXX = 2940.038	LXY = 0.000	LXZ = -0.000
LYX = 0.000	LYY = 2940.141	LYZ = 5.374
LZX = -0.000	LZY = 5.374	LZZ = 0.558

Section Modulus:

From the calculated Second Moment of Inertia from SolidWorks, the section modulus of the aluminum support extrusions can be found.

$$S_a = \frac{I_{y_a}}{c_a}$$
 $S_a = \frac{.330in^4}{1.025in} = .3219in^3$

b. Winchester Top Rail:



Section properties of the selected face of EPN 0566

Area = 0.565 inches²

Centroid relative to output coordinate system origin: (inches) $\begin{array}{l} X=0.000\\ Y=0.141\\ Z=69.000 \end{array}$

Moments of inertia of the area, at the centroid: (inches 4)Lxx = 0.150Lxy = 0.000Lxz = 0.000

Lyx = 0.000	Lyy = 0.311	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 0.461

Polar moment of inertia of the area, at the centroid = 0.461 inches ^ 4

Angle between principal axes and part axes = 0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.150 Iy = 0.311

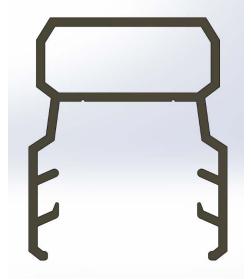
Moments of inertia of the a	rea, at the output coordinate s	ystem: (inches ^ 4)
LXX = 2690.239	LXY = 0.000	LXZ = 0.000
LYX = 0.000	LYY = 2690.389	LYZ = 5.491
LZX = 0.000	LZY = 5.491	LZZ = 0.472

Section Modulus:

From the calculated Second Moment of Inertia from SolidWorks, the section modulus of the aluminum support extrusions can be found.

$$S_b = \frac{I_{yb}}{c_b}$$
 $S_b = \frac{0.311in^4}{1.05in} = 0.2962in^3$

c. Avalon Tristan Top Rail:



Section properties of the selected face of EPN 0507

Area = 0.5806 inches^2

Centroid relative to output coordinate system origin: (inches)

X = 0.0000 Y = -0.1586Z = 70.2500

Moments of inertia of the area, at the centroid: (inches 4)

Lxx = 0.2284	Lxy = 0.0000	Lxz = 0.0000
Lyx = 0.0000	Lyy = 0.2412	Lyz = 0.0000
Lzx = 0.0000	Lzy = 0.0000	Lzz = 0.4696

Polar moment of inertia of the area, at the centroid = 0.4696 inches 4

Angle between principal axes and part axes = -0.0000 degrees

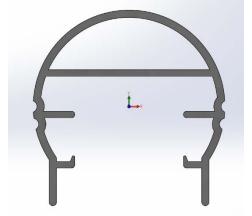
Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.2284 Iy = 0.2412

Section Modulus:

From the calculated Second Moment of Inertia from SolidWorks, the section modulus of the aluminum support extrusions can be found.

$$S_c = \frac{I_{yc}}{C_c}$$
 $S_c = \frac{0.2412 in^4}{0.85 in} = 0.2838 in^3$

d. Avalon Lanval Top Rail:



Section properties of the selected face of EPN 0518

```
Area = 0.644 inches^2
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Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.140 Z = 71.500

Moments of inertia of the area, at the centroid: (inches 4) Lxx = 0.214 Lxy = 0.000 Lxz = 0.000

	01211		01000		01000
Lyx =	0.000	Lyy =	0.356	Lyz =	0.000
Lzx =	0.000	Lzy =	0.000	Lzz =	0.570

Polar moment of inertia of the area, at the centroid = 0.570 inches ^ 4

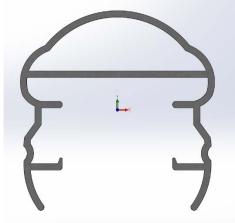
Angle between principal axes and part axes = -0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) \$Ix=0.214\$\$\$ Iy=0.356\$

Section Modulus:

$$S_d = \frac{l_{yd}}{C_d} S_d = \frac{0.356 \, in^4}{1.048 in} = 0.3397 in^3$$

e. Avalon Pellinore Top Rail:



Section properties of the selected face of EPN 0519

```
Area = 0.623 inches^2
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Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.139Z = 71.500

Moments of inertia of the area, at the centroid: (inches ^ 4)

Lxx = 0.213	Lxy = 0.000	Lxz = 0.000
Lyx = 0.000	Lyy = 0.358	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 0.571

Polar moment of inertia of the area, at the centroid = 0.571 inches 4

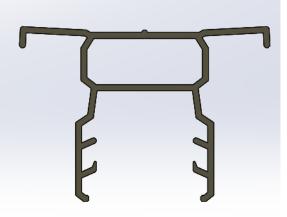
Angle between principal axes and part axes = -0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) \$Ix=0.213\$\$\$ Iy=0.358\$

Section Modulus:

$$S_e = \frac{I_{ye}}{C_e}$$
 $S_e = \frac{0.358 \, in^4}{1.05 in} = 0.3410 in^3$

f. Avalon Oberon Top Rail:



Section properties of the selected face of EPN 0519

Area = 0.737 inches^2

Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.026Z = 71.500

Moments of inertia of the area, at the centroid: (inches ^ 4)

Lxx = 0.322	Lxy = 0.000	Lxz = 0.000
Lyx = 0.000	Lyy = 0.465	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 0.787

Polar moment of inertia of the area, at the centroid = 0.571 inches 4

Angle between principal axes and part axes = -0.000 degrees

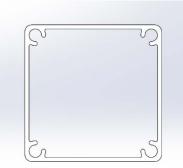
Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.322 Iy = 0.465

Section Modulus:

$$S_f = \frac{l_{yf}}{c_f}$$
 $S_f = \frac{0.465 \, in^4}{1.5415 in} = 0.3016 in^3$

b. Structural Posts

1. Residential Post



Section properties of the selected face of EPN 0504

Area = 0.83 inches^2

Centroid relative to output coordinate system origin: (inches)

X = 0.00 Y = 0.00Z = 39.00

Moments of inertia of the area, at the centroid: (inches ^ 4)

Lxx = 0.82 $Lxy = 0.00$ $Lxz = 0.0$	
)
Lyx = 0.00 Lyy = 0.82 Lyz = 0.0)
Lzx = 0.00 $Lzy = 0.00$ $Lzz = 1.6$	3

Polar moment of inertia of the area, at the centroid = 1.63 inches 4

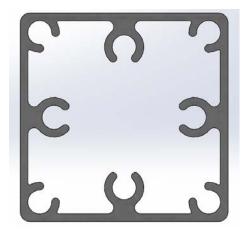
Angle between principal axes and part axes = -0.00 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.82 Iy = 0.82

Section Modulus:

$$S_p = \frac{I_{yp}}{c_p} \qquad \qquad S_{p1} = \frac{0.82in^4}{1.25in} = 0.656in^3$$

2. Over The Top Post



Section properties of the selected face of EPN 0541

Area = 0.972 inches^2

Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.000Z = 12.000

Moments of inertia of the area, at the centroid: (inches ^ 4)
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Lxx = 0.502	Lxy = 0.000	Lxz = 0.000
Lyx = 0.000	Lyy = 0.502	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 1.003

Polar moment of inertia of the area, at the centroid = 0.017 inches ^ 4

Angle between principal axes and part axes = -0.000 degrees

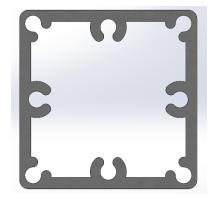
Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.502 Iy = 0.502

Section Modulus:

$$S_P = rac{I_P}{C_P}$$

 $S_P = rac{.502in^4}{1.00in} = .502in^3$

3. Commercial Post



Area = 1.782 inches^2

Centroid relative to output coordinate system origin: (inches) $X\,=\,0.000$

Y = 0.000Z = 60.000

Moments of inertia of the area, at the cent	roid: (inches ^ 4)	
Lxx = 1.468	Lxy = -0.000	Lxz = 0.000
Lyx = -0.000	Lyy = 1.468	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 2.937

Polar moment of inertia of the area, at the centroid = 2.937 inches ^ 4

Angle between principal axes and part axes = 79.210 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) $Ix=1.468 \\ Iy=1.468$

Moments of inertia of the area, at the output coordinate system: (inches ^ 4)				
LXX = 6417.478	LXY = 0.000	LXZ = -0.000		
LYX = 0.000	LYY = 6417.478	LYZ = -0.000		
LZX = -0.000	LZY = -0.000	LZZ = 2.937		

Section Modulus:

$$S_{p} = \frac{I_{yp}}{c_{p}}$$
$$S_{p2} = \frac{1.468in^{4}}{1.25in} = 1.1744in^{3}$$

c. Infill (Balusters)

1. Square Baluster



Section properties of the selected face of EPN 0506

Area = 0.103 inches^2

Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.000Z = 33.000

Polar moment of inertia of the area, at the centroid = 0.017 inches 4

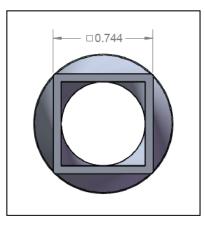
Angle between principal axes and part axes = -0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.009 Iy = 0.009

Section Modulus:

$$S_b = \frac{I_{yb}}{cb}$$
 $S_b = \frac{0.009in^4}{0.372in} = 0.024in^3$

2. Twisted Baluster



Section properties of the selected face of EPN 0563

Area = 0.161 inches²

Centroid relative to output coordinate system origin: (inches)

X = 0.000 Y = 0.000 Z = 33.000

Moments of inertia of the area, at the centroid: (inches ^ 4)

Lxx = 0.013	Lxy = 0.000	Lxz = 0.000
Lyx = 0.000	Lyy = 0.013	Lyz = 0.000
Lzx = 0.000	Lzy = 0.000	Lzz = 0.025

Polar moment of inertia of the area, at the centroid = 0.025 inches ^ 4

Angle between principal axes and part axes = 0.000 degrees

Principal moments of inertia of the area, at the centroid: (inches ^ 4) Ix = 0.013Iy = 0.013

Moments of inertia of the	e area, at the output coordinate sy	stem: (inches ^ 4)
LXX = 175.686	LXY = 0.000	LXZ = 0.000
LYX = 0.000	LYY = 175.686	LYZ = 0.000
LZX = 0.000	LZY = 0.000	LZZ = 0.025

Section Modulus:

$$S_b = \frac{I_{yb}}{cb} \qquad S_b = \frac{0.013in^4}{0.372in} = 0.0349in^3$$

III. <u>Structural Post Load Analysis</u>

Load Distribution

Before analysis can be completed on the structural posts of a metal railing system, it must be determined if a Load Proportion Factor can be applied. In certain circumstances, a post is allowed a percent reduction in total load resistance requirements, assuming that adjacent posts and rails will take a percentage of any applied load. This reduction is called the Load Proportion Factor (Pf). The percentage is determined by the ratio of the stiffness of the rail (Kr) relative to the stiffness of the post (Kp). Calculation of the Stiffness Ratio (Rr) and the corresponding Load Proportion Factor can be seen below.

Stiffness Ratio:

$$R_r = \frac{K_r}{K_p}$$

The stiffness of the rail can be determined using the modulus of elasticity (E) of Aluminum, the second moment of inertia (I^*) and the span length (L) of the rail:

$$K_r = \frac{E_r \cdot I_r}{L_r} \qquad \qquad E_r = 10e6\,psi$$

69" Cambridge Rail Section

93" Cambridge Rail Section

69" Winchester Rail Section

93" Winchester Rail Section

$$K_a = \frac{10e6psi \cdot 0.330in^4}{69in} = 47,826.08in$$

$$K_b = \frac{10e6psi \cdot 0.330in^4}{93in} = 35,483.87in$$

$$K_c = \frac{10e6psi \cdot 0.311in^4}{69in} = 45,072.46in$$

$$K_d = \frac{10e6psi \cdot 0.311in^4}{93in} = 33,440.86in$$

6' Avalon Tristan Rail Section

$$K_e = \frac{10e6\,psi\cdot.2412in^4}{72in} = 33,500.00in$$

$$K_f = \frac{10e6\,psi\cdot.2412in^4}{96in} = 25,125.00in$$

8' Avalon Tristan Rail Section

6' Avalon Lanval Rail Section

$$K_g = \frac{10e6 \ psi \cdot .356in^4}{72in} = 49,444.44in$$

8' Avalon Lanval Rail Section

6' Avalon Pellinore Rail Section

8' Avalon Pellinore Rail Section

6' Avalon Oberon Rail Section

$$K_h = \frac{10e6\,psi\cdot.356in^4}{96in} = 37,083.33in$$

$$K_i = \frac{10e6 \ psi \cdot .358in^4}{72in} = 49,722.22in$$

$$K_j = \frac{10e6 \ psi \cdot .358in^4}{96in} = 37,291.67in$$

$$K_k = \frac{10e6\,psi\cdot.465in^4}{72in} = 64,583.33in$$

8' Avalon Oberon Rail Section

$$K_l = \frac{10e6\,psi\cdot.465in^4}{96in} = 48,437.5in$$

Stiffness of Post:

The stiffness of the post can be determined the same way using the height of the post instead of the length of the section and the Modulus of Elasticity. When performing these calculations, the height (h) of the post represents the tallest mounted height of the railing. See "Calculation of Post Section Modulus" for calculation of Post second moment of Inertia.

$$K_p = \frac{E_p \cdot I_p}{h_p}$$
 $E_p = 10e6\,psi$ $K_p = \frac{10e6\,psi \cdot 1.58in^4}{42in} = 376,190.48in$

93" Cambridge Rail Section Worst Case

$$\therefore R_a = \frac{K_b}{K_p} = \frac{35,483.87in}{376,190.48in} = .094$$

93" Winchester Rail Section Worst Case

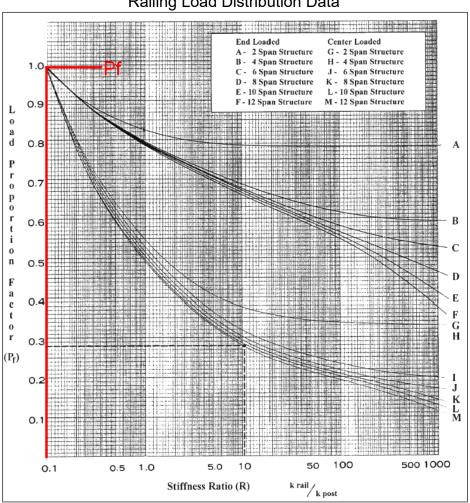
$$\therefore R_b = \frac{K_d}{K_p} = \frac{33,440.86in}{376,190.48in} = .088$$

6' Avalon Rail Section Worst Case

$$\therefore R_c = \frac{K_c}{K_p} = \frac{33,500in}{376,190.48in} = 0.089$$

8' Avalon Rail Section Worst Case

$$\therefore R_d = \frac{K_d}{K_p} = \frac{25,125in}{376,190.48in} = 0.067$$



Railing Load Distribution Data

The structural post analysis can now be completed without the application of a Load Proportion Factor to show the worst case scenario.

According to metal railing requirements, the bending moment (BM) on a post is defined as the horizontal load multiplied by the height of the load. This value can not be greater than the resisting moment (RM), which is defined as the allowable design stress multiplied by the section modulus.

Equations for post analysis:

 $BendingMoment: BM = P \cdot h$ ResistingMoment : $RM = \sigma_y \cdot S$ $\therefore BM \leq RM$

Using the section modulus of the post in the Barrette Aluminum Railing system, it can be shown that the post more than exceeds the requirements of the governing equations.

Determination of appropriate Bending Moment:²

The analysis will be calculated considering the two possible scenarios, 36" and 42" heights.

$$\sigma_{yp} = 34,800 \, psi$$

$$\Omega_b = 1.67 \text{ (AISC F1)}$$

According to the 2015 IBC, a guard rail of this type must undergo a 200 lb point load (P) at any point on the top rail and a 50lb/ft distributed load along the top rail. Under a 50lb/ft load, a 96" rail section is exposed to 400lbs of force. In terms of the post of such a rail system, each supporting post on a section would take half of this applied load, or 200lb. With consecutive sections, any one post could be exposed to the loading of two sections. In this scenario, as much as a 400lb applied load could be experienced. This case shall be examined as it represents the largest possible bending moment.

For residential 36" applications, only the point load is required. Posts are sold in lengths of 39", so worst case scenario is calculated according to a point load at the very top of the post. Over the top posts are sold in 33" lengths so the finished rail height (36") is used to calculate the bending moment. For commercial 42" applications, uniform loading is the worst case scenario and occurs at the finished rail height of 42".

Residential 39" Post:

$$BendingMoment(BM) = P \cdot h_p = 200lb \cdot 39in = 7,800.00inlb$$

Resisting Moment (RM) = $\sigma_{yp} \cdot S_{p1} = (34,800psi \cdot 0.656in^3) = 22,829$ inlb

$$\therefore RM > BM$$

Resulting Factor of Safety:

$$FOS = \frac{RM}{BM} = \frac{22,828inlb}{7,800inlb} = 2.93 > 1.67 \text{ DESIGN OK}$$

² Railing Dynamics Inc. recommends the use of thru bolts as a means of fastening to mounting surfaces. Because of the varying degrees of material strength regarding decking structure material, the attachment method is not considered in this analysis.

Over the Top 33" Post (OTT):

The over the top system is offered in both residential and commercial applications. Residential applications have a maximum span of 6 ft while commercial spans are limited to 5 ft.

Residential:

Bending
$$Moment(BM) = P \cdot h_p = 200lb \cdot 36in = 7,200$$
 inlb

Resisting Moment (RM) = $\sigma_{yp} \cdot S_{p1} = (34,800psi \cdot 0.502in^3) = 17,469.6$ inlb

$$\therefore RM > BM$$

Resulting Factor of Safety:

$$FOS = \frac{RM}{BM} = \frac{17,469.6inlb}{7,200inlb} = 2.43 > 1.67 \text{ DESIGN OK}$$

Commercial OTT:

$$P = w \cdot l = 50lbft \cdot 5ft = 250 \, lb$$

Bending Moment(BM) = $P \cdot h_p = 250 \ lb \cdot 36in = 9,000 \ inlb$

Resisting Moment (RM) = $\sigma_{yp} \cdot S_{p1} = (34,800psi \cdot 0.502in^3) = 17,469.6$ inlb

$$\therefore RM > BM$$

Resulting Factor of Safety:

$$FOS = \frac{RM}{BM} = \frac{17,469.6inlb}{9,000inlb} = 1.94 > 1.67 \text{ DESIGN OK}$$

Commercial 45" Post:

$$BendingMoment(BM) = P \cdot h_p = 400lb \cdot 42in = 16,800.00inlb$$

Resisting Moment (RM) = $\sigma_{yp} \cdot S_{p1} = (34,800psi \cdot 1.174in^3) = 40,855.2$ inlb

 $\therefore RM > BM$

Resulting Factor of Safety:

$$FOS = \frac{RM}{BM} = \frac{40,855.2inlb}{16,800inlb} = 2.43 > 1.67 \text{ DESIGN OK}$$

With the above calculations, it is proven that the resisting moment in the post is larger than the bending moment caused by the load, showing that the posts in the Barrette Aluminum Railing system exceed design requirements.

Maximum Post Spacing:

According to the 2015 IBC, a metal railing system must be able to withstand a 50lb/ft uniform load (w) at the highest point on the rail in any direction. With this value and the geometric properties of the aluminum posts, the maximum post spacing can be calculated.

$$L_r = \frac{\sigma_{yp} \cdot S_p}{\Omega_b(w/12) \cdot h_p} \qquad \qquad L_r = \frac{34,800 \, psi \cdot 1.1744 in^3}{1.67 \cdot (50/12) \cdot 42 in} = 139.84 in$$

The maximum span in a Barrette Aluminum Railing System is 93". With this calculation, it is clear that the post more than exceeds the railing span requirements.

IV. Guard Rails

Maximum Concentrated Load:

For railing systems required to experience a concentrated load, the maximum moment and thus determining point, occurs at midspan. This moment is determined by the load and the span length and is resisted by the allowable stress multiplied by the section modulus.

For simply supported rail with the point load:

Bending Moment(BM) =
$$\frac{P \cdot L}{4}$$

Resisting Moment (RM) = $\sigma_{yr} \cdot S_r$

 $\sigma_{yr} = 34,800 \, psi$ $\Omega_b = 1.67 \, (AISC \, F1)$

In order to calculate allowable point force, bending moment and resisting moment needs to be compared:

$$\frac{P \cdot L}{4} = \sigma_{yr} \cdot S_r$$

Rearranging the above equation, allowable load "P" can be calculated

$$P = \frac{\sigma_{yr} \cdot S_r \cdot 4}{L}$$

Resulting Factor of Safety:

$$if FOS = \frac{P}{200 \text{lbs}} > 1.67 \text{ then } \text{DESIGN OK}$$

200lbs load is required by ASTM. To meet or exceed 1.67 ratio (FOS) P has to meet or exceed 334lbs.

a. 93" Cambridge Railing:

$$P_a = \frac{34,800psi \cdot 0.3219in^3 \cdot 4}{93in} = 481.8lbs$$
$$FOS(P_a) = \frac{481.8lbs}{200lbs} = 2.41 > 1.67 \text{ DESIGN OK}$$

b. 93" Winchester Railing:

$$P_b = \frac{34,800psi \cdot 0.2962in^3 \cdot 4}{93in} = 443..3lbs$$
$$FOS(P_b) = \frac{443.3lbs}{200lbs} = 2.21 > 1.67 \text{ DESIGN OK}$$

c. 8' Avalon Rail - Tristan:

$$P_c = \frac{34,800psi \cdot 0.2838in^3 \cdot 4}{96in} = 411.5lbs$$

$$FOS(P_c) = \frac{411.5lbs}{200lbs} = 2.06 > 1.67 \text{ DESIGN OK}$$

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d. 8' Avalon Rail - Lanval:

$$P_d = \frac{34,800psi \cdot 0.3397in^3 \cdot 4}{96in} = 492.6lbs$$
$$FOS(P_d) = \frac{492.6lbs}{200lbs} = 2.46 > 1.67 \text{ DESIGN OK}$$

e. 8' Avalon Rail - Pellinore:

$$P_e = \frac{34,800psi \cdot 0.3410in^3 \cdot 4}{96in} = 494.5lbs$$
$$FOS(P_e) = \frac{494.5lbs}{200lbs} = 2.47 > 1.67 \text{ DESIGN OK}$$

f. 8' Avalon Rail – Oberon

$$P_f = \frac{34,800psi \cdot 0.3016in^3 \cdot 4}{96in} = 437.3.5lbs$$
$$FOS(P_f) = \frac{437.3lbs}{200lbs} = 2.19 > 1.67 \text{ DESIGN OK}$$

According to the ASTM standard E 985, "Standard Specification for Permanent Metal Railing Systems and Rails for Buildings", the maximum required concentrated load the Barrette Aluminum Railing System is 200lb. With this calculation, it is clear that the rail more than exceeds this railing load requirement.

Maximum Uniform Load:

For simply supported rail with uniform distributed load:

Bending Moment(BM) =
$$\frac{w \cdot L^2}{8}$$

Resisting Moment (RM) =
$$\sigma_{yr} \cdot S_r$$

 $\sigma_{yr} = 34,800 \, psi$ $\Omega_b = 1.67 \, (AISC \, F1)$

In order to calculate allowable point force, bending moment and resisting moment needs to be compared:

$$\frac{w \cdot L^2}{8} = \sigma_{yr} \cdot S_r$$

Rearranging the above equation, allowable load "w" can be calculated

$$w = \frac{\sigma_{yr} \cdot S_r \cdot 8}{L^2} \cdot 12 \frac{in}{ft}$$

Resulting Factor of Safety:

$$if FOS(w) = \frac{w}{50 \text{lbs}} > 1.67 \text{ then } DESIGN \text{ ok}$$

50lbs load is required by ASTM. To meet or exceed 1.67 ratio (FOS) "w" has to meet or exceed 83.5lbs.

a. 93" Cambridge Railing:

$$w_a = \frac{34800psi \cdot 0.3219in^3 \cdot 8}{(93in)^2} \cdot 12\frac{in}{ft} = 124.34lb/ft$$
$$FOS(w_a) = \frac{124.34lbs}{50lbs} = 2.49 > 1.67 \text{ DESIGN OK}$$

b. 93" Winchester Railing:

$$w_b = \frac{34800psi \cdot 0.2962in^3 \cdot 8}{(93in)^2} \cdot 12\frac{in}{ft} = 114.41lb/ft$$
$$FOS(w_b) = \frac{114.41lbs}{50lbs} = 2.29 > 1.67 \text{ DESIGN OK}$$

c. 8' Avalon Rail - Tristan:

$$w_c = \frac{34800psi \cdot 0.2838in^3 \cdot 8}{(96in)^2} \cdot 12\frac{in}{ft} = 102.88lb/ft$$

$$FOS(w_c) = \frac{102.88lbs}{50lbs} = 2.06 > 1.67 \text{ DESIGN OK}$$

d. 8' Avalon Rail - Lanval:

$$w_{d} = \frac{34800psi \cdot 0.3397in^{3} \cdot 8}{(96in)^{2}} \cdot 12\frac{in}{ft} = 123.14lb/ft$$
$$FOS(w_{d}) = \frac{123.14lbs}{50lbs} = 2.46 > 1.67 \text{ DESIGN OK}$$

e. 8' Avalon Rail - Pellinore:

$$w_e = \frac{34800psi \cdot 0.3410in^3 \cdot 8}{(96in)^2} \cdot 12\frac{in}{ft} = 123.61lb/ft$$
$$FOS(w_e) = \frac{123.61lbs}{50lbs} = 2.47 > 1.67 \text{ DESIGN OK}$$

f. 8' Avalon Rail - Oberon:

$$w_f = \frac{34800psi \cdot 0.3016in^3 \cdot 8}{(96in)^2} \cdot 12\frac{in}{ft} = 109.33lb/ft$$
$$FOS(w_f) = \frac{109.33lbs}{50lbs} = 2.19 > 1.67 \text{ DESIGN OK}$$

According to the ASTM standard E 985, "Standard Specification for Permanent Metal Railing Systems and Rails for Buildings", the maximum required uniform load is 50lb/ft. With this calculation, it is clear that the rail more than exceeds this railing load requirement.

V. Infill

The infill is required to resist a 50lb. load applied over 12" x 12" square. This load will be spread over 2 balusters, exhibiting a force of 25 lbs. on each baluster.

For simply supported picket with the point load:

Bending Moment(BM) =
$$\frac{P \cdot L}{4}$$

Resisting Moment (RM) = $\sigma_{yr} \cdot S_r$

 $\Omega_b = 1.67 (AISC F1)$

In order to calculate allowable point force, bending moment and resisting moment needs to be compared:

$$\frac{P \cdot L}{4} = \sigma_{yr} \cdot S_r$$

Rearranging the above equation, allowable load "P" can be calculated

$$P = \frac{\sigma_{yr} \cdot S_r \cdot 4}{L}$$

Resulting Factor of Safety:

$$if FOS = \frac{P}{25lbs} > 1.67 then DESIGN OK$$

25lbs load (per picket) is required by ASTM. To meet or exceed 1.67 ratio (FOS) P has to meet or exceed 41.75lbs.

Square Baluster:

Material: AI-6063-T5, $\sigma_{vr} = 21,000 \, psi$

$$P_a = \frac{21,000psi \cdot 0.024in^3 \cdot 4}{39in} = 51.69lbs$$

$$FOS(P_a) = \frac{51.69lbs}{25lbs} = 2.07 > 1.67$$
 DESIGN OK

Square Baluster:

Material: Al-6063-T6, $\sigma_{yr} = 21,000 \, psi$

$$P_a = \frac{21,000psi \cdot 0.024in^3 \cdot 4}{39in} = 51.69lbs$$

$$FOS(P_a) = \frac{51.69lbs}{25lbs} = 2.07 > 1.67$$
 DESIGN OK

Twisted Baluster:

Material: Al-6063-T6, $\sigma_{yr} = 31,000 \ psi$

$$P_a = \frac{31,000psi \cdot 0.0349in^3 \cdot 4}{33in} = 131,14lbs$$
$$FOS(P_a) = \frac{131.14lbs}{25lbs} = 5.25 > 1.67 \text{ DESIGN OK}$$

Glass Infill:

Material: AFG Standard Float Glass

Glass infill is compliant per IBC Section 2407.1 at finished rail heights of 36" and 42" (See appendix for Barrette material spec).

VI. System Assembly Fastener Considerations

The posts of the Barrette Aluminum Railing System are attached to the base plate custom screws part 34109436 (See appendix for screw information). Four screws are used for residential applications and eight are used for commercial applications. The screws have a minimum minor diameter of 0.185in. The withdrawal of these screws have been tested and approved through third party testing. ATI test report "B8389.01-119-19" conducted on 10/31/12 shows a similar railing system being tested using the same posts and fasteners for this system. From this testing and data, it can be seen that the post and fastener assembly exceeded both code and design requirements.

In addition, the maximum shear load resistance the fasteners must exhibit occurs when a load is applied directly adjacent to the mounting plate. At this point, bending of the post is negligible and all force is being transmitted through the bracket fasteners in the form of shear force.

Residential Post Application:

Screw material: C1022

Safety factor for screw - 3

 $\sigma_{TU} = 55,000 \ psi$

 $\sigma_S = 0.6 * \sigma_{TU} = 0.6 * 55,000 \ psi = 33,060 \ psi$

$$V = \tau A$$
 $\tau = \frac{V}{A}$ $A = \pi (r)^2 = \pi (\frac{D}{2})^2$ $V = \frac{F}{4}$ (4 fasteners)

$$A = \pi \left(\frac{0.185in}{2}\right)^2 = 0.02688in^2$$
$$V = \frac{200lb}{4} = 50lb$$

$$\tau = \frac{50lb}{0.02688in^2} = 1,860.12\,psi$$

We determine the Factor of Safety using the yield stress for a C1022 steel proof load:

$$FOS(\tau) = \frac{\sigma_s}{\tau}$$

 $\sigma_S = 33,060 psi$

$$FOS(t) = \frac{33,060psi}{1,860.12psi} = 17.77 > 3 DESIGN OK$$

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Commercial Post Application:

$$V = \tau A \qquad \tau = \frac{V}{A} \qquad A = \pi (r)^2 = \pi (\frac{D}{2})^2 \qquad V = \frac{F}{8} \text{ (8 fasteners)}$$
$$A = \pi (\frac{0.185in}{2})^2 = 0.02688in^2$$
$$V = \frac{400lb}{8} = 50lb$$
$$\tau = \frac{50lb}{0.02688in^2} = 1,860.12\,psi$$

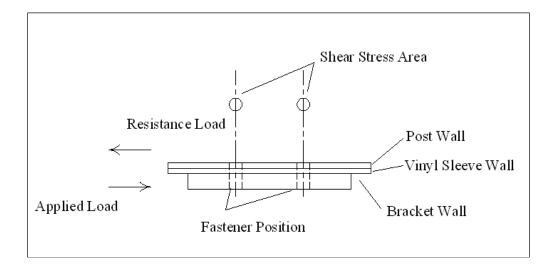
We determine the Factor of Safety using the yield stress for a 410 Stainless steel proof load:

$$FOS(\tau) = \frac{\sigma_s}{\tau} \qquad \sigma_s = 42,100 \, psi$$
$$FOS(\tau) = \frac{42,100 \, psi}{1,860.12 \, psi} = 22.63$$

Guardrail Application:

The guard rails of the Barrette Aluminum Railing System are attached to the posts with self-tapping 410 stainless steel screws with Aluminum ADC12 brackets. The screws have a minimum minor diameter of 0.1in.

The maximum load resistance the fasteners must exhibit occurs when a 200lb load is applied directly adjacent to the post. At this point, bending of the guard rail is negligible and all force is being transmitted through the bracket fasteners in the form of shear force. A shear force diagram and calculation can be seen below.



$$V = \tau A$$
 $\tau = \frac{V}{A}$ $A = \pi (r)^2 = \pi (\frac{D}{2})^2$ $V = \frac{F}{4}$ (4 fasteners)

$$A = \pi (\frac{0.1in}{2})^2 = 0.00785in^2$$
$$V = \frac{200lb}{4} = 50lb$$

$$\tau = \frac{50lb}{0.00785in^2} = 6,369.4\,psi$$

We determine the Factor of Safety using the yield stress for a 410 Stainless steel proof load:

 $\sigma_s = 42,100 \, psi$ $FOS(\tau) = \frac{42,100 \, psi}{6,369.4 \, psi} = 6.6$

Impact Consideration:

A general calculation can be done to show the ability of the fasteners to withstand an impact load, as if a person was to fall into the railing system.

Consider the weight of an average man being dropped onto a horizontally mounted railing system being modeled as a beam supported by stiff springs. The weight will be dropped from a height so as to represent a distance one would travel in a trip and fall type scenario. With this information, an impact factor can be calculated and applied to the stress experienced in normal static loading conditions. Assumptions:

Falling distance: 36in Average male weight: 190lbs Impact occurs at mid span

Impact Factor Calculation:

$$I.F._{(r)} = 1 + \sqrt{1 + \frac{2h}{\delta_t}} \qquad \qquad \delta_t = \delta_b + \delta_s \qquad \qquad \delta_{br} = \frac{PLr^3}{48EI_{yr}} \qquad \qquad \delta_{sr} = \frac{P}{2k}$$

h= height at which load is dropped, 36 in.

- δ_{h} = beam deflection
- δ_s = spring deflection ~ 0
- a. 93" Cambridge Railing:

$$\delta_{ba} = \frac{PL_a^3}{48EI_{ya}} = \frac{190lb \cdot (93in)^3}{48 \cdot 10e^6 psi \cdot .330in^4} = .9648in$$
$$\delta_{sa} = \frac{P}{2k} = \frac{190}{2 \cdot \infty} = 0in$$
$$\delta_{ta} = \delta_{ba} = .9648in$$
$$I.F._{(a)} = 1 + \sqrt{1 + \frac{2 \cdot 36in}{.9648in}} = 9.69$$

This factor can now be applied to the loading scenario and compared to the maximum load resistance of the fastening screws.

Assumptions:

Because the force is being applied at the mid span of the beam, its value can be decreased by half to represent equal support on each end.

$$V_{a} = \frac{\frac{F}{2} \cdot I.F._{(a)}}{4} = \frac{\frac{190lb}{2} \cdot 9.69}{4} = 230.138lb \text{ (adjust for 4 fasteners in each bracket)}$$
$$\tau_{a} = \frac{V_{a}}{A} = \frac{230.138lb}{0.00785in^{2}} = 29,316.9\,psi$$

$$\sigma_s = 42,100 \, psi$$
 (410 stainless steel proof load)

$$FOS(\tau_a) = \frac{\sigma_p}{\tau_a} = \frac{42,100\,psi}{29,316.9} = 1.43$$

a. 93" Winchester Railing:

$$\delta_{ba} = \frac{PL_{b^{3}}}{48EI_{yb}} = \frac{190lb \cdot (93in)^{3}}{48 \cdot 10e^{6}psi \cdot .311in^{4}} = 1.023in$$
$$\delta_{sa} = \frac{P}{2k} = \frac{190}{2 \cdot \infty} = 0in$$
$$\delta_{ta} = \delta_{ba} = 1.023in$$
$$I.F._{(a)} = 1 + \sqrt{1 + \frac{2 \cdot 36in}{1.023in}} = 9.448$$

This factor can now be applied to the loading scenario and compared to the maximum load resistance of the fastening screws.

Assumptions:

Because the force is being applied at the mid span of the beam, its value can be decreased by half to represent equal support on each end.

 $V_{a} = \frac{\frac{F}{2} \cdot I.F._{(a)}}{4} = \frac{\frac{190lb}{2} \cdot 9.448}{4} = 224.39lb \text{ (adjust for 4 fasteners in each bracket)}$ $\tau_{a} = \frac{V_{a}}{A} = \frac{224.39lb}{0.00785in^{2}} = 28,584.7\,psi$ $\sigma_{s} = 42,100\,psi \text{ (410 stainless steel proof load)}$

$$FOS(\tau_a) = \frac{\sigma_p}{\tau_a} = \frac{42,100\,psi}{28,584.7} = 1.47$$

VII. Post Surface Connection Details:

The force at the bottom of the post/plate must be calculated and then compared to the resisting moment and shear exerted by the fasteners. The same calculation method will be used for the analysis for residential and commercial structural posts. The structure supporting the post and the connection from the post to the structure must be designed to resist the listed shear load(S) and the listed tension/compression load(F). The design of the supporting structure and the connection to the structure are the responsibility of others. Sample connection shown below for illustration purposes only.

From section III:

Residential 36" Post: $BendingMoment(BM) = P \cdot h_p = 200lb \cdot 39in = 7,800.00inlb = 650lb - ft$

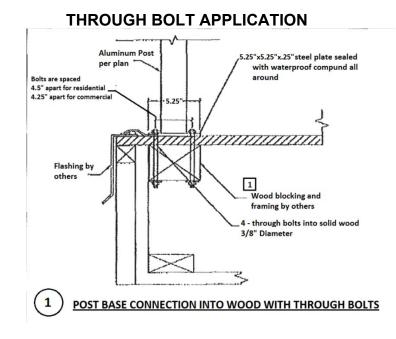
Commercial 42" Post:

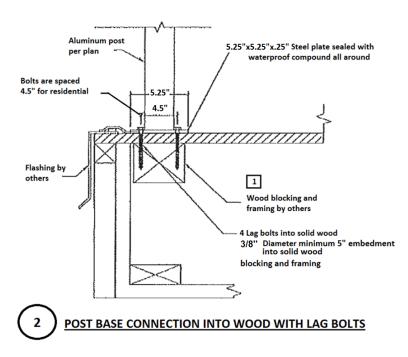
 $BendingMoment(BM) = P \cdot h_p = 400lb \cdot 42in = 16,800.00inlb = 1,400lb - ft$

This results in the force on the bottom of the baseplate as follows:

Residential – mounting baseplate 34115035 (See Appendix for details) F = BM / d = 650lb / .3 = 2,167.00lb - ft = 2.2KipsShear (S) = P = 200lb

Commercial – mounting baseplate 34115034 (See Appendix for details) F = BM / d = 1,500lb / .3 = 1,400.00lb - ft = 4,667lbs = 4.7KipsShear (S) = P = 400lb





LAG SCREW APPLICATION

VIII. Conclusion

With the above analysis, it can be proven that the Barrette Aluminum Railing system, more than exceeds the design requirements set forth by the applicable sections of the 2018 IBC and 2018 IRC. This analysis is also valid for the 2009,2012 & 2015IBC/ IRC

The engineering department at Railing Dynamics, Inc. gives its full endorsement of the Barrette Aluminum Railing system as a qualified safety guardrail system

- IX. Appendix
 - A. Material Properties
 - a. Aluminum 6005-T5
 - b. Aluminum 6063-T5
 - c. Aluminum 6063-T6
 - d. Tensile Strength of Bolts
 - e. Withdrawal Design Values
 - B. ATI Test Report
 - C. Barrette Outdoor Living Parts
 - a. EPN-0504 2.5" Post
 - b. EPN-0527 2.5" Heavy Duty Post
 - c. 34115036 Post Mount Plate (4 Holes)
 - d. 34115035 Post Mount Plate (8 Holes)
 - e. 34115034 Post Mount Plate (12 Holes)
 - f. 34109436 Custom Screw
 - g. 34114816 Bottom Bracket
 - h. 34114819 Bottom Bracket Cover
 - i. EPN-0566 Winchester Rail Channel
 - j. EPN-0565 Cambridge Rail Channel
 - k. EPN-0513 Avalon U-Channel
 - I. 34114813 Top Bracket
 - m. EPN-0506 Square Baluster
 - n. EPN-0563 Twisted Baluster
 - o. 34114807 Rail Support Foot
 - p. 34114810 Rail Support Foot Cover
 - q. 34107308 Glass Baluster 36" Rail
 - r. EPN-0507 Tristan Rail
 - s. EPN-0518 Lanval Rail
 - t. EPN-0519 Pellinore Rail
 - u. EPN-0743 Oberon Rail
 - v. EPN-0541 2x2 Post (Over the Top)
 - w. 34110275 Glass Baluster 42" Rail

With cooperation with:

Chris Schneider Senior Engineer Railing Dynamics, Inc. 135 Steelmanville Rd Egg Harbor Township, NJ 08234 web: www.rdirail.com phone: 800-488-7245

Appendix A

Aluminum 6005-T5			
Categories:	Metal; Nonferrous Metal; Aluminum Alloy; 6000 Series Aluminum Alloy		
Material Notes:	Data points with the AA note have been provided by the Aluminum Association, Inc. and are NOT FOR DESIGN.		
	Composition Notes : Composition information provided by the Aluminum Association and is not for design.		
Key Words:	UNS A96005; ISO AlSiMg; Aluminium 6005-T5; AA6005-T5		

Vendors: No vendors are listed for this material. Please <u>click here</u> if you are a supplier and would like information on how to add your listing to this material.

Physical Properties	Metric	English	Comments
Density	2.70 g/cc	0.0975 lb/in ³	AA; Typical
Mechanical Properties	Metric	English	Comments
Hardness, Brinell	95	95	500 kg load with 10 mm ball
Hardness, Knoop	120	120	Converted from Brinell Hardness Value
Hardness, Rockwell A	39.8	39.8	Converted from Brinell Hardness Value
Hardness, Rockwell B	60	60	Converted from Brinell Hardness Value
Hardness, Vickers	107	107	Converted from Brinell Hardness Value
Tensile Strength, Ultimate	260 MPa	37700 psi	
Tensile Strength, Yield	240 MPa	34800 psi	
Elongation at Break	8.0 % @Thickness 1.60 mm	8.0 % @Thickness 0.0630 in	In 5 cm
Modulus of Elasticity	69.0 GPa	10000 ksi	Average of Tension and Compression. In Aluminum alloys, the compressive modulus is typically 2% greater than the tensile modulus
Poissons Ratio	0.33	0.33	Estimated from trends in similar Al alloys.
Fatigue Strength	100 MPa @# of Cycles 5.00e+8	14500 psi @# of Cycles 5.00e+8	
Shear Modulus	26.0 GPa	3770 ksi	Estimated from similar Al alloys.
Shear Strength	205 MPa	29700 psi	
Electrical Properties	Metric	English	Comments
Electrical Resistivity	0.00000349 ohm-cm @Temperature 20.0 °C	0.00000349 ohm-cm @Temperature 68.0 °F	AA; Typical
Thermal Properties	Metric	English	Comments
CTE, linear 🌆	23.4 µm/m-°C @Temperature 20.0 - 100 °C @	13.0 µin/in-°F Temperature 68.0 - 212 °F	AA; Typical; average over range

	25.0 μm/m-°C @Temperature 20.0 - 300 °C (13.9 µin/in-°F @Temperature 68.0 - 572 °F	
Specific Heat Capacity	0.890 J/g-°C	0.213 BTU/lb-°F	Estimated from trends in similar Al alloys.
Thermal Conductivity	189 W/m-K	1310 BTU-in/hr-ft ² -°F	AA; Typical at 77°F
Melting Point	607.2 - 654 °C	1125 - 1210 °F	AA; Typical range based on typical composition for wrought products 1/4 inch thickness or greater; Eutectic melting can be completely eliminated by homogenization.
Solidus	607.2 °C	1125 °F	AA; Typical
Liquidus	654 °C	1210 °F	AA; Typical
Processing Properties	Metric	English	Comments
Annealing Temperature	414 °C	778 °F	hold at temperature for 2 to 3 hr
Solution Temperature	546.1 °C	1015 °F	
Aging Temperature	174 °C	346 °F	hold at temperature for 8 hr
Component Elements Properties	Metric	English	Comments
Aluminum, Al	97.5 - 99 %	97.5 - 99 %	As remainder
Chromium, Cr	<= 0.10 %	<= 0.10 %	
Copper, Cu	<= 0.10 %	<= 0.10 %	
Iron, Fe	<= 0.35 %	<= 0.35 %	
Magnesium, Mg	0.40 - 0.60 %	0.40 - 0.60 %	
Manganese, Mn	<= 0.10 %	<= 0.10 %	
Other, each	<= 0.05 %	<= 0.05 %	
Other, total	<= 0.15 %	<= 0.15 %	
Silicon, Si	0.60 - 0.90 %	0.60 - 0.90 %	
Titanium, Ti	<= 0.10 %	<= 0.10 %	
Zinc, Zn	<= 0.10 %	<= 0.10 %	

References for this datasheet.

Some of the values displayed above may have been converted from their original units and/or rounded in order to display the information in a consistent format. Users requiring more precise data for scientific or engineering calculations can click on the property value to see the original value as well as raw conversions to equivalent units. We advise that you only use the original value or one of its raw conversions in your calculations to minimize rounding error. We also ask that you refer to MatWeb's terms of use regarding this information. Click here to view all the property values for this datasheet as they were originally entered into MatWeb.

Aluminum 6063-T5

Categories:	Metal; Nonferrous Metal; Aluminum Alloy; 6000 Series Aluminum Alloy		
Material Notes:	Applications include pipe, railings, furniture, architectural extrusions, irrigation pipes, and transportation.		
	Data points with the AA note have been provided by the Aluminum Association, Inc. and are NOT FOR DESIGN.		
	Composition Notes: Composition information provided by the Aluminum Association and is not for design.		
Key Words:	UNS A96063; ISO AlMg0.5Si; Aluminium 6063-T5; AA6063-T5		

Vendors: No vendors are listed for this material. Please <u>click here</u> if you are a supplier and would like information on how to add your listing to this material.

Physical Properties	Metric	English	Comments
Density	2.70 g/cc	0.0975 lb/in ³	AA; Typical
Mechanical Properties	Metric	English	Comments
Hardness, Brinell	60	60	AA; Typical; 500 g load; 10 mm ball
Hardness, Knoop	83	83	Converted from Brinell Hardness Value
Hardness, Vickers	70	70	Converted from Brinell Hardness Value
Tensile Strength, Ultimate	186 MPa	27000 psi	AA; Typical
th	16.0 MPa @Temperature 371 °C	2320 psi @Temperature 700 °F	
	23.0 MPa @Temperature 316 °C	3340 psi @Temperature 601 °F	
	31.0 MPa @Temperature 260 °C	4500 psi @Temperature 500 °F	
	62.0 MPa @Temperature 204 °C	8990 psi @Temperature 399 °F	
	138 MPa @Temperature 149 °C	20000 psi @Temperature 300 °F	
	165 MPa @Temperature 100 °C	23900 psi @Temperature 212 °F	
	186 MPa @Temperature 24.0 °C	27000 psi @Temperature 75.2 °F	
	193 MPa @Temperature -28.0 °C	28000 psi @Temperature -18.4 °F	
	200 MPa @Temperature -80.0 °C	29000 psi @Temperature -112 °F	
	255 MPa @Temperature -196 °C	37000 psi @Temperature -321 °F	
Tensile Strength, Yield	145 MPa	21000 psi	AA; Typical

th

Aluminum 6063-T5

	14.0 MPa @Strain 0.200 %,	2030 psi @Strain 0.200 %,	
	Temperature 371 °C	Temperature 700 °F	
	17.0 MPa @Strain 0.200 %, Temperature 316 °C	2470 psi @Strain 0.200 %, Temperature 601 °F	
	24.0 MPa @Strain 0.200 %,	3480 psi @Strain 0.200 %,	
	Temperature 260 °C 45.0 MPa	Temperature 500 °F 6530 psi	
	@Strain 0.200 %, Temperature 204 °C	@Strain 0.200 %, Temperature 399 °F	
	124 MPa @Strain 0.200 %, Temperature 149 °C	18000 psi @Strain 0.200 %, Temperature 300 °F	
	138 MPa @Strain 0.200 %, Temperature 100 °C	20000 psi @Strain 0.200 %, Temperature 212 °F	
	145 MPa @Strain 0.200 %, Temperature 24.0 °C	21000 psi @Strain 0.200 %, Temperature 75.2 °F	
	152 MPa @Strain 0.200 %, Temperature -80.0 °C	22000 psi @Strain 0.200 %, Temperature -112 °F	
	152 MPa @Strain 0.200 %, Temperature -28.0 °C	22000 psi @Strain 0.200 %, Temperature -18.4 °F	
	165 MPa @Strain 0.200 %, Temperature -196 °C	23900 psi @Strain 0.200 %, Temperature -321 °F	
Elongation at Break <u>III</u>	18 % @Temperature 100 °C	18 % @Temperature 212 °F	
	20 % @Temperature 149 °C	20 % @Temperature 300 °F	
	22 % @Temperature 24.0 °C	22 % @Temperature 75.2 °F	
	23 % @Temperature -28.0 °C	23 % @Temperature -18.4 °F	
	24 % @Temperature -80.0 °C 28 %	24 % @Temperature -112 °F 28 %	
	@Temperature -196 °C 40 %	@Temperature -321 °F 40 %	
	@Temperature 204 °C 75 %	@Temperature 399 °F 75 %	
	@Temperature 260 °C 80 %	@Temperature 500 °F 80 %	
	@Temperature 316 °C 105 %	@Temperature 601 °F 105 %	
	@Temperature 371 °C 12 %	@Temperature 700 °F 12 %	AA; Typical
Modulus of Elasticity	@Thickness 1.59 mm 68.9 GPa	@Thickness 0.0625 in 10000 ksi	AA; Typical; Average of tension and compression. Compression modulus is about 2% greater than tensile modulus.
Poissons Ratio	0.33	0.33	
Fatigue Strength	68.9 MPa @# of Cycles 5.00e+8	10000 psi @# of Cycles 5.00e+8	completely reversed stress; RR Moore machine/specimen
Shear Modulus	25.8 GPa	3740 ksi	
Shear Strength	117 MPa	17000 psi	AA; Typical

Comments	English	Metric	Electrical Properties
AA; Typical	0.00000316 ohm-cm @Temperature 68.0 °F	0.00000316 ohm-cm @Temperature 20.0 °C	Electrical Resistivity
Comments	English	Metric	Thermal Properties
	12.1 µin/in-°F @Temperature -58.0 - 68.0 °F	21.8 µm/m-°C @Temperature -50.0 - 20.0 °C	CTE, linear 🌆
AA; Typical; average over range	13.0 µin/in-°F @Temperature 68.0 - 212 °F	23.4 µm/m-°C @Temperature 20.0 - 100 °C	
	13.6 µin/in-°F @Temperature 68.0 - 392 °F	24.5 μm/m-°C @Temperature 20.0 - 200 °C	
	14.2 µin/in-°F @Temperature 68.0 - 572 °F	25.6 µm/m-°C @Temperature 20.0 - 300 °C	
	0.215 BTU/lb-°F	0.900 J/g-°C	Specific Heat Capacity
AA; Typical at 77°F	1450 BTU-in/hr-ft ² -°F	209 W/m-K	Thermal Conductivity
AA; Typical range based on typical composition for wrought products 1/4 inch thickness or greater	1140 - 1210 °F	616 - 654 °C	Melting Point
AA; Typical	1140 °F	616 °C	Solidus
AA; Typical	1210 °F	654 °C	Liquidus
Comments	English	Metric	Processing Properties
hold at temperature for 2 to 3 hr; cool at 50° F per hour from 775 to 500°F	775 °F	413 °C	Annealing Temperature
	970 °F	521 °C	Solution Temperature
hold at temperature for 1 hr	360 °F	182 °C	, Aging Temperature
hold at temperature for 1 hr	400 °F	204 °C	i oniporatorio
Comments	English	Metric	Component Elements Properties
As remainder	<= 97.5 %	<= 97.5 %	Aluminum, Al
	<= 0.10 %	<= 0.10 %	Chromium, Cr
	<= 0.10 %	<= 0.10 %	Copper, Cu
	<= 0.35 %	<= 0.35 %	Iron, Fe
	0.45 - 0.90 %	0.45 - 0.90 %	Magnesium, Mg
	<= 0.10 %	<= 0.10 %	Manganese, Mn
	<= 0.05 %	<= 0.05 %	Other, each
	<= 0.15 %	<= 0.15 %	Other, total
	0.20 - 0.60 %	0.20 - 0.60 %	Silicon, Si
	<= 0.10 %	<= 0.10 %	Titanium, Ti

References for this datasheet.

Aluminum 6063-T6			
Categories:	Metal; Nonferrous Metal; Aluminum Alloy; 6000 Series Aluminum Alloy		
Material Notes:	Applications include pipe, railings, furniture, architectural extrusions, irrigation pipes, and transportation.		
	Data points with the AA note have been provided by the Aluminum Association, Inc. and are NOT FOR DESIGN.		
	Composition Notes : Composition information provided by the Aluminum Association and is not for design.		
Key Words:	UNS A96063; ISO AlMg0.5Si; Aluminium 6063-T6; AA6063-T6		
Vendors:	Click here to view all available suppliers for this material.		
	Please click here if you are a supplier and would like information on how to add your listing to		

Please <u>click here</u> if you are a supplier and would like information on how to add your listing to this material.

Physical Properties	Metric	English	Comments
Density	2.70 g/cc	0.0975 lb/in³	AA; Typical
Mechanical Properties	Metric	English	Comments
Hardness, Brinell	73	73	AA; Typical; 500 g load; 10 mm ball
Hardness, Knoop	96	96	Converted from Brinell Hardness Value
Hardness, Vickers	83	83	Converted from Brinell Hardness Value
Tensile Strength, Ultimate	241 MPa	35000 psi	AA; Typical
	16.0 MPa @Temperature 371 °C 23.0 MPa @Temperature 316 °C 31.0 MPa @Temperature 260 °C 62.0 MPa @Temperature 204 °C 145 MPa @Temperature 149 °C 214 MPa @Temperature 100 °C 241 MPa @Temperature 24.0 °C 248 MPa @Temperature -28.0 °C 262 MPa @Temperature -80.0 °C	2320 psi @Temperature 700 °F 3340 psi @Temperature 601 °F 4500 psi @Temperature 500 °F 8990 psi @Temperature 399 °F 21000 psi @Temperature 300 °F 31000 psi @Temperature 212 °F 35000 psi @Temperature 75.2 °F 36000 psi @Temperature -18.4 °F 38000 psi @Temperature -112 °F	
	324 MPa @Temperature -196 °C 214 MPa	47000 psi @Temperature -321 °F 31000 psi	AA; Typical

http://www.matweb.com/search/datasheet_print.aspx?matguid=333b3a557aeb49b2b17266... 6/18/2015

Tensile Strength, Yield			
ш.	14.0 MPa @Strain 0.200 %, Temperature 371 °C	2030 psi @Strain 0.200 %, Temperature 700 °F	
	17.0 MPa @Strain 0.200 %, Temperature 316 °C	2470 psi @Strain 0.200 %, Temperature 601 °F	
	24.0 MPa @Strain 0.200 %, Temperature 260 °C	3480 psi @Strain 0.200 %, Temperature 500 °F	
	45.0 MPa @Strain 0.200 %, Temperature 204 °C	6530 psi @Strain 0.200 %, Temperature 399 °F	
	133 MPa @Strain 0.200 %, Temperature 149 °C	19300 psi @Strain 0.200 %, Temperature 300 °F	
	193 MPa @Strain 0.200 %, Temperature 100 °C	28000 psi @Strain 0.200 %, Temperature 212 °F	
	214 MPa @Strain 0.200 %, Temperature 24.0 °C	31000 psi @Strain 0.200 %, Temperature 75.2 °F	
	221 MPa @Strain 0.200 %, Temperature -28.0 °C	32100 psi @Strain 0.200 %, Temperature -18.4 °F	
	228 MPa @Strain 0.200 %, Temperature -80.0 °C	33100 psi @Strain 0.200 %, Temperature -112 °F	X
	248 MPa @Strain 0.200 %, Temperature -196 °C	36000 psi @Strain 0.200 %, Temperature -321 °F	
Elongation at Break <mark>II</mark> I	15 % @Temperature 100 °C	15 % Temperature 212 °F@	
	18 % @Temperature 24.0 °C	18 % @Temperature 75.2 °F	
	19 % @Temperature -28.0 °C 20 %	19 % @Temperature -18.4 °F 20 %	
	20 % @Temperature -80.0 °C 20 %	20 % @Temperature -112 °F 20 %	
	@Temperature 149 °C 24 %	@Temperature 300 °F 24 %	
	@Temperature -196 °C 40 %	@Temperature -321 °F 40 %	
	@Temperature 204 °C 75 %	@Temperature 399 °F 75 %	
	@Temperature 260 °C 80 %	@Temperature 500 °F 80 %	
	@Temperature 316 °C 105 %	@Temperature 601 °F 105 %	
	@Temperature 371 °C 12 %	@Temperature 700 °F 12 %	AA; Typical
	@Thickness 1.59 mm	@Thickness 0.0625 in	
Modulus of Elasticity	68.9 GPa	10000 ksi	AA; Typical; Average of tension and compression. Compression modulus is about 2% greater than tensile modulus.
Ultimate Bearing Strength	434 MPa	62900 psi	Edge distance/pin diameter = 2.0

Bearing Yield Strength	276 MPa	40000 psi	Edge distance/pin diameter = 2.0
Poissons Ratio	0.33	0.33	
Fatigue Strength	68.9 MPa @# of Cycles 5.00e+8	10000 psi @# of Cycles 5.00e+8	completely reversed stress; RR Moore machine/specimen
Machinability	50 %	50 %	0-100 Scale of Aluminum Alloys
Shear Modulus	25.8 GPa	3740 ksi	
Shear Strength	152 MPa	22000 psi	AA; Typical
Electrical Properties	Metric	English	Comments
Electrical Resistivity	0.00000332 ohm-cm @Temperature 20.0 °C	0.00000332 ohm-cm @Temperature 68.0 °F	AA; Typical
Thermal Properties	Metric	English	Comments
CTE, linear 📶	21.8 µm/m-°C @Temperature -50.0 - 20.0 °C	12.1 µin/in-°F @Temperature -58.0 - 68.0 °F	
	23.4 µm/m-°C @Temperature 20.0 - 100 °C	13.0 µin/in-°F @Temperature 68.0 - 212 °F	AA; Typical; average over range
	24.5 μm/m-°C @Temperature 20.0 - 200 °C	13.6 µin/in-°F @Temperature 68.0 - 392 °F	
	25.6 µm/m-°C @Temperature 20.0 - 300 °C	14.2 μin/in-°F @Temperature 68.0 - 572 °F	
Specific Heat Capacity	0.900 J/g-°C	0.215 BTU/lb-°F	
Thermal Conductivity	200 W/m-K	1390 BTU-in/hr-ft ² -°F	AA; Typical at 77°F
Melting Point	616 - 654 °C	1140 - 1210 °F	AA; Typical range based on typical composition for wrought products 1/4 inch thickness or greater
Solidus	616 °C	1140 °F	AA; Typical
Liquidus	654 °C	1210 °F	AA; Typical
Processing Properties	Metric	English	Comments
Annealing Temperature	413 °C	775 °F I	hold at temperature for 2 to 3 hr; cool at 50° F per hour from 775 to 500°F
Solution Temperature	521 °C	970 °F	
Aging Temperature	177 °C	350 °F	hold at temperature for 8 hr
Component Elements Properties	Metric	English	Comments
Aluminum, Al	<= 97.5 %	<= 97.5 %	As remainder
Chromium, Cr	<= 0.10 %	<= 0.10 %	
Copper, Cu	<= 0.10 %	<= 0.10 %	
Iron, Fe	<= 0.35 %	<= 0.35 %	
Magnesium, Mg	0.45 - 0.90 %	0.45 - 0.90 %	
Manganese, Mn	<= 0.10 %	<= 0.10 %	

Other, each	<= 0.05 %	<= 0.05 %
Other, total	<= 0.15 %	<= 0.15 %
Silicon, Si	0.20 - 0.60 %	0.20 - 0.60 %
Titanium, Ti	<= 0.10 %	<= 0.10 %
Zinc, Zn	<= 0.10 %	<= 0.10 %

References for this datasheet.

Some of the values displayed above may have been converted from their original units and/or rounded in order to display the information in a consistent format. Users requiring more precise data for scientific or engineering calculations can click on the property value to see the original value as well as raw conversions to equivalent units. We advise that you only use the original value or one of its raw conversions in your calculations to minimize rounding error. We also ask that you refer to MatWeb's terms of use regarding this information. Click here to view all the property values for this datasheet as they were originally entered into MatWeb.

			-		lable		nsile Its, I	_			
Nominal B	iolt D	iameter d	l _b , in.	5	/8	3	/4	7	/8		1
Nomina	al Bol	it Area, in	2	0.3	107	0.4	142	0.6	i01	0.7	785
ASTM Desi	a.	F _{nt} /Ω (ksi)	¢F _{nt} (ksi)	r _n /Ω	ф г , .	r _n/Ω	¢r _n	r _o /Ω	¢ r n	r _n /Ω	¢r _n
	-	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
A325 & F18	52	45.0	67.5	13.8	20.7	19.9	29.8	27.1	40.6	35.3	53.0
A490		56.5	84.8	17.3	26.0	25.0	37.4	34.0	51.0	44.4	66.6
A307		22.5	33.8	6.90	10.4	9.94	14.9	13.5	20.3	17.7	26.5
Nominal B	olt D	iameter o	l _b , in.	11	/8	1	1/4	13	/8	11	/2
Nomina	al Bol	t Area, in	,2	0.9	94	1.	23	17	48	1.	77
ASTM Desi	Q.	F _{nt} /Ω (ksi)	¢ <i>F_{nt}</i> (ksi)	r _n /Ω	ф г п	r _n /Ω	фг _п	r _η/Ω	фг _л	r _n /Ω	φ r _n
	-	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
A325 & F18	52	45.0	67.5	44.7	67.1	55.2	82.8	66.8	100	79.5	119
A490		56.5	84.8	56.2	84.2	69.3	104	83.9	126	99.8	150
A307		22.5	33.8	22.4	33.5	27.6	41.4	33.4	50.1	39.8	59.6
ASD	I	.RFD									
$\Omega_{\gamma} = 2.00$	¢,	= 0.75									

11.2 Reference Withdrawal Design Values

11.2.1 Lag Screws

11.2.1.1 The reference withdrawal design values, in lb/in. of penetration, for a single lag screw inserted in side grain, with the lag screw axis perpendicular to the wood fibers, shall be determined from Table 11.2A or Equation 11.2-1, within the range of specific gravities and screw diameters given in Table 11.2A. Reference withdrawal design values, W, shall be multiplied by all applicable adjustment factors (see Table 10.3.1) to obtain adjusted withdrawal design values, W'.

 $W = 1800 G^{3/2} D^{3/4}$ (11.2-1)

11.2.1.2 When lag screws are loaded in withdrawal from end grain, reference withdrawal design values, W, shall be multiplied by the end grain factor, $C_{eg} = 0.75$.

1605

11.2.1.3 When lag screws are loaded in withdrawal, the tensile strength of the lag screw at the net (root) section shall not be exceeded (see 10.2.3).

Table 11.2A Lag Screw Reference Withdrawal Design Values (W)¹

Tabulated withdrawal design values (W) are in pounds per inch of thread penetration into side grain of main member. Length of thread penetration in main member shall not include the length of the tapered tip (see Appendix L).

Specific Gravity,		<u></u>		Lag Scre	w Unthr	eaded Sh	ank Dian	neter, D			
G	1/4"	5/16"	3/8"	7/16"	1/2"	5/8"	3/4"	7/8"	1"	1-1/8"	1-1/4"
0.73	397	469	538	604	668	789	905	1016	1123	1226	1327
0.71	381	450	516	579	640	757	868	974	1077	1176	1273
0.68	357	422	484	543	600	709	813	913	1009	1103	1193
0.67	349 😳		473	s 531	∞587	694	796	893	987	1078	1167
0.58	281	332	381	428	473	559	641	719	795	869	940
0.55	260	207	- 352	395	437	516	592	664	734	802	868
0.51	232	274	314	353	390	461	528	593	656	716	775
0.50	225	266	(305)	342	378	447	513	576	636	695	752
0.49	218	258	296	332	367	434	498	559	617	674	730
0.47	205	242	278	312	345	408	467	525	580	634	686
0.46	199	235	269	302	334	395	453	508	562	613	664
0.44	186	220	252	283	312	369	423	475	525	574	621
0.43	179	212	243	273	302	357	409	459	508	554	600
0.42	173	205	235	264_	291	344	395	443	490	535	579
0.41	167	198	226	254	281	332	381	428	473	516	559
. 0.40	161	190	218	245	271	320	367	412	455	497	538
0.39	155	183	210	236	261	308	353	397	438	479	518
0.38	3.149	176	202	227	251	296	340	381 🚍	422	461	498
0.37	143	169	194	218	241	285	326	367	405	443	479
0.36	A ALANCE A	163	186	209	231	273	313	352	389	425	460
0.35	132	156	179	200	222	262	300	337	373	407	441
0.31	1	130	149	167	185	218	250	281	''311 ''	339	367

1. Tabulated withdrawal design values (W) for lag screw connections shall be multiplied by all applicable adjustment factors (see Table 10.3.1).

Appendix B



TEST REPORT

Rendered to:

BARRETTE OUTDOOR LIVING, INC.

For:

VersaRail Aluminum Guardrail Assembly

Report No: B8389.01-119-19 Report Date: 10/31/12

130 Derry Court York, PA 17406-8405 phone: 717-764-7700 fax: 717-764-4129 www.archtest.com



TEST REPORT

B8389.01-119-19 October 31, 2012

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TEST REPORT

Rendered to:

BARRETTE OUTDOOR LIVING, INC. 740 North Main Street Bulls Gap, Tennessee 37711

 Report No.:
 B8389.01-119-19

 Test Started:
 04/20/12

 Test Completed:
 04/20/12

 Report Date:
 10/31/12

1.0 General Information

1.1 Product

VersaRail Aluminum Guardrail Assembly

1.2 Project Description

Architectural Testing was contracted by Barrette Outdoor Living Inc. to conduct structural performance tests on their *VersaRail* aluminum guardrail assembly in a level application. The assembly was evaluated for the design load requirements of the following building codes:

2012 International Residential Code[®], International Code Council

Structural tests were performed according to Chapter 17 (Structural Tests and Special Inspections) of IBC 2012.

1.3 Limitations

All tests performed were to evaluate structural performance of the guardrail assembly to carry and transfer imposed loads to the supporting structure. The test specimens evaluated included the infill, rails, rail brackets, and support posts. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

1.4 Qualifications

Architectural Testing has demonstrated compliance with ANS/ISO/IEC Standard 17025 and is consequently accredited as a Testing Laboratory (TL-144) by International Accreditation Service, Inc.

130 Derry Court York, PA 17406-8405 phone: 717-764-7700 fax: 717-764-4129 www.archtest.com



1.5 Product Description

Barrette Outdoor Living provided the partially-assembled test specimens with details as listed below. All extruded parts were 6005-T5, 6063-T5 or 6063-T6 alloy aluminum, and all cast parts were AA380.0-F aluminum. See drawings in Appendix A and photographs in Appendix B for additional details.

<u>Top Rail Cap</u>: 2-1/8 in high by 1-3/4 in wide contoured aluminum extrusion with 0.07 in wall

Top Sub-rail and Bottom Rail: 1-3/16 in wide by 1 in deep U-shaped aluminum extrusion with 0.07 in wall

Pickets: 3/4 in square, hollow aluminum extrusion with 0.04 in wall

<u>Picket Locking Strip</u>: 3/4 in wide by 0.07 in thick polypropylene extrusion located in bottom and top sub-rail

Rail Brackets: Cast aluminum socket brackets contoured to shape of rails

- <u>Fasteners</u>: #8 x 1-1/4 in (18-TPI, 0.162 in major dia., 0.120 in minor dia.) hex head, self-starting, sheet metal screw (four in top bracket / post and two in bottom bracket / post); #8 x 2 in (18-TPI, 0.165 in major dia., 0.113 in minor dia.) pan head, square drive, self-starting, carbon steel screw (two in top bracket / rail)
- Posts: 2-1/2 in square by 0.07 in thick extruded aluminum attached to a 5-1/4 in square by 0.25 in thick AISI 1010 steel base plate with four 1/4 in by 2-1/2 in (20-TPI, 0.245 in major dia., 0.185 in minor dia., type F point) trim head, phillips drive, stainless steel screws.

2.0 Structural Performance Testing of Assembled Railing Systems

2.1 Test Equipment

The guardrail was tested in a self-contained structural frame designed to accommodate anchorage of the guardrail assembly and application of the required test loads. The specimens were loaded using an electric winch mounted to a rigid steel test frame. High strength steel cables, nylon straps, and load distribution beams were used to impose test loads on the specimens. Applied load was measured using an electronic load cell located in-line with the loading system. Electronic linear displacement transducers were used to measure deflections.



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2.2 Test Setup

The 8 ft by 36 in *VersaRail* aluminum guardrail assembly was installed and tested as a single railing section by directly securing the posts into a simulated mock wood deck with four 5/16 in Gr. 5 bolts with washers and nuts. Additional wood blocking was added to the simulated wood deck per the manufacturer's instructions. See blocking instructions in Appendix C for additional information. Transducers mounted to an independent reference frame were located to record movement of reference points on the guardrail system components (ends and mid-point) to determine net component deflections. See photographs in Appendix B for individual test setups.

2.3 Test Procedure

Each test specimen was inspected prior to testing to verify size and general condition of the materials, assembly, and installation. No potentially compromising defects were observed prior to testing. An initial load, not exceeding 50% of design load, was applied and transducers were zeroed. Load was then applied at a steady uniform rate until reaching 2.0 times design load in no less than 10 seconds. After reaching 2.0 times design load, the load was released. After allowing a minimum period of one minute for stabilization, load was reapplied to the initial load level used at the start of the loading procedure, and deflections were recorded and used to analyze recovery. Load was then increased at a steady uniform rate until reaching 2.5 times design load or until failure occurred. The testing time was continually recorded from the application of initial test load until the ultimate test load was reached.

2.4 Test Results

The following tests were performed on the guardrail assemblies for the design load requirements of the referenced codes. Deflection and permanent set were component deflections relative to their end-points; they were not overall system displacements. All loads and displacement measurements were horizontal, unless noted otherwise.

Key to Test Results Tables:

Load Level: Target test load

Test Load: Actual applied load at the designated load level (target).

<u>Elapsed Time (E.T.)</u>: The amount of time into the test with zero established at the beginning of the loading procedure.



2.4 Test Results (Continued)

Test Series No. 1 93-1/2 in by 36 in *VersaRail* Aluminum Level Guardrail System Limited to IRC – One- and Two-Family Dwellings

		Specimen No. 1	l of 1					
Dosigu	n Load: 50 lb / 1 S	Fest No. 1 – 04/		Fill (Two]	Pickets)			
		E.T.		icket Displ	0.2	n)		
Load Level	Test Load (lb)	(min:sec)	End	Mid	End	Net 1		
Initial Load	26	00:00	0.00	0.00	0.00	0.00		
2.0 x Design Load	101	00:16	0.42	0.97	1.14	0.19		
Initial Load	26	01:52	0.00	0.00	0.04	0.00		
	100% Rec	covery from 2.0	x Design L	oad				
2.5 x Design Load								
		10. N. N.	2042.223					

¹ Net displacement was the picket displacement relative to its top and bottom.

Design	Load: 50 lb / 1 S	Fest No. 2 – 04/ Square Ft at Bo		Fill (Two	Pickets)		
		Е.Т.	Bottom Rail Displacement (in)				
Load Level	Test Load (lb)	(min:sec)	End	Mid	End	Net ¹	
Initial Load	25	00:00	0.00	0.00	0.00	0.00	
2.0 x Design Load	101	00:25	0.11	1.72	0.13	1.60	
Initial Load	25	02:08	0.00	0.01	0.01	0.01	
	99% Rec	overy from 2.0	x Design Lo	bad			
2.5 x Design Load	126	02:49	Ach	ieved Load	without Fa	ilure	

¹ Net displacement was the bottom rail displacement relative to its ends.

Test No. 3 – 04/20/12 Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail								
E.T. Rail Displacemen								
Load Level	Test Load (lb)	(min:sec)	End	Mid	End	Net ¹		
Initial Load	80	00:00	0.00	0.00	0.00	0.00		
2.0 x Design Load	401	00:56	1.10	2.89	0.77	1.96		
Initial Load	80	02:47	0.15	0.20	0.09	0.08		
	96% Rec	overy from 2.0	x Design Lo	oad				
2.5 x Design Load	500	03:51	Ach	ieved Load	without Fa	ilure		

¹ Net displacement was mid-rail displacement relative to the support posts.



2.4 Test Results (Continued)

x 17 1		E.T.	Displacement (in)			
Load Level	Test Load (lb)	(min:sec)	Post #1 Post #			
Initial Load	101	00:00	0.00	0.00		
2.0 x Design Load	802	01:04	2.60	2.02		
Initial Load	101	02:45	0.57	0.38		
	78% Post #1 Red	covery from 2.0 x	Design Load			
	81% Post #2 Rec	covery from 2.0 x	Design Load			
2.5 x Design Load	1002	04:07	Achieved Load	without Failure		

Test Series No. 1 (Continued)

¹ Transducers were mounted to the posts.

2.5 Summary and Conclusions

Using performance criteria of withstanding an ultimate load of 2.5 times design load, the test results met the test load requirements (design load plus factor of safety of two and one-half) of the referenced building codes for the nominal 8 ft wide by 36 in high *VersaRail* aluminum guardrail assembly and its support posts reported herein. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.



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3.0 Closing Statement

Detailed drawings, data sheets, representative samples of test specimens, a copy of this test report, and all other supporting evidence will be retained by Architectural Testing for a period of four years from the original test date. At the end of this retention period, said materials shall be discarded without notice, and the service life of this report by Architectural Testing shall expire. Results obtained are tested values and were secured using the designated test methods. This report neither constitutes certification of this product nor expresses an opinion or endorsement by this laboratory; it is the exclusive property of the client so named herein and relates only to the tested specimens. This report may not be reproduced, except in full, without the written approval of Architectural Testing.

For ARCHITECTURAL TESTING:

Iv Signed by: Kyle Evans

Kyle J. Evans Technician II Structural Systems Testing

KJE:vtm/drm

Virgal T. Mickley, Jr., P.E. Senior Project Engineer Structural Systems Testing

Attachments (pages): This report is complete only when all attachments listed are included.
 Appendix A - Drawings (14)
 Appendix B - Photographs (4)
 Appendix C - Blocking Instructions (1)



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Revision Log

Rev. # Date Page(s)

0

10/31/12 N/A

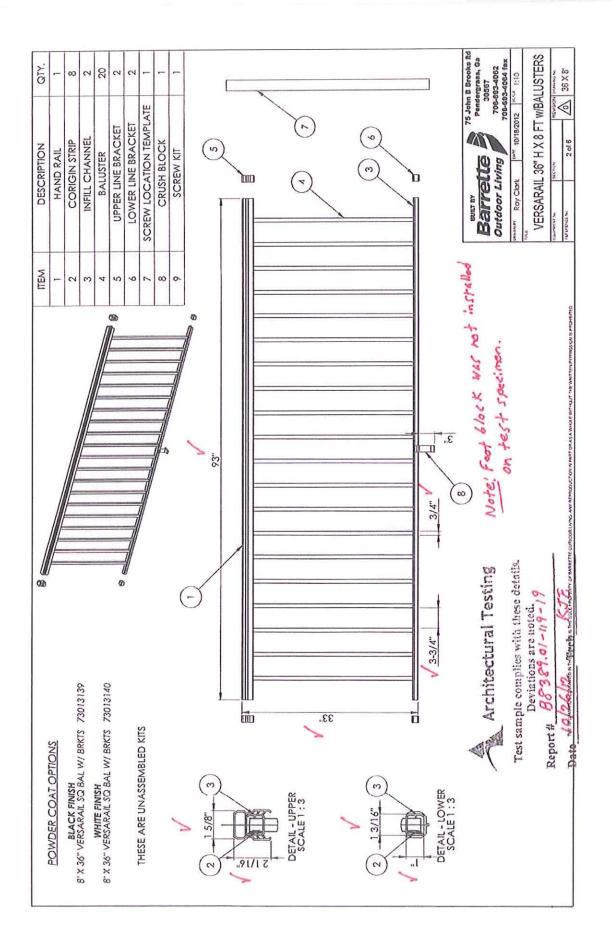
Revision(s)
Original report issue

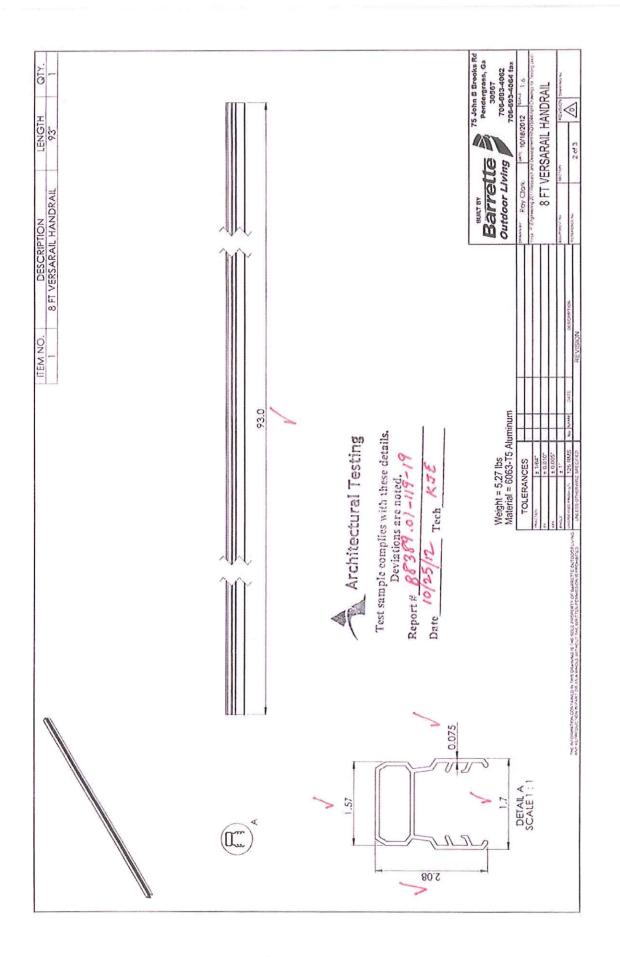


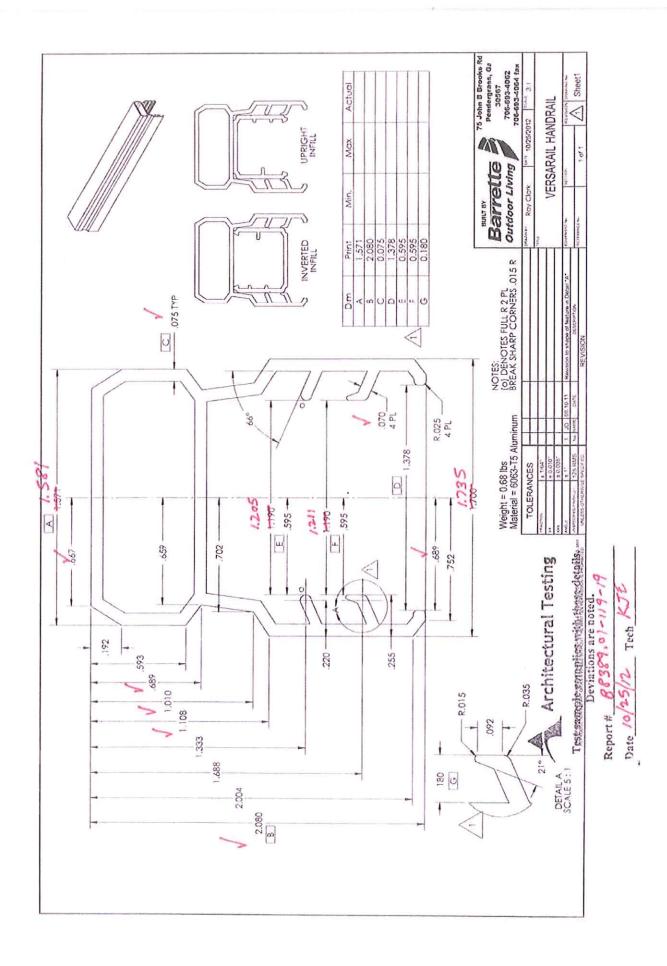
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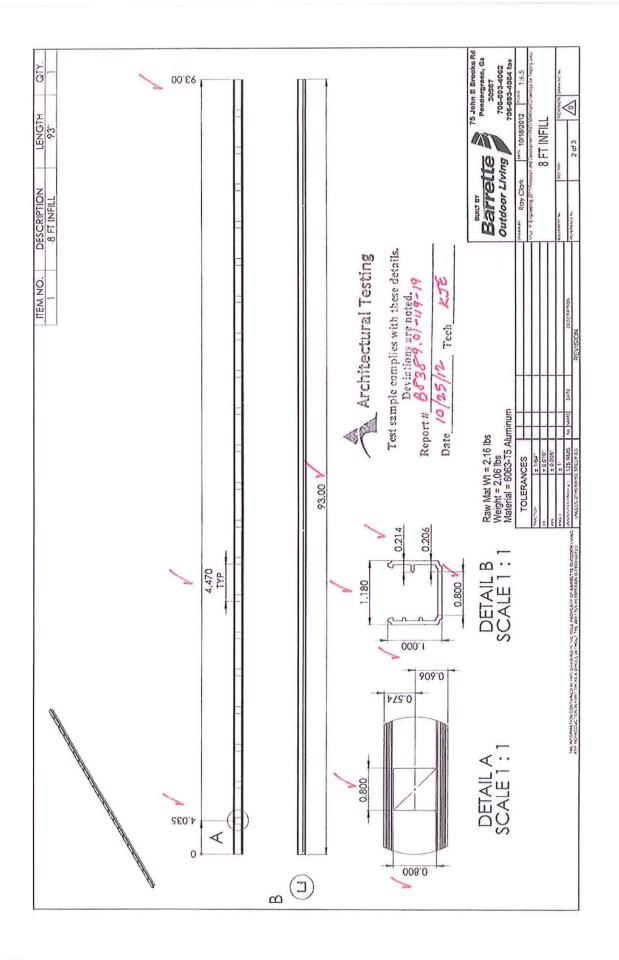
APPENDIX A

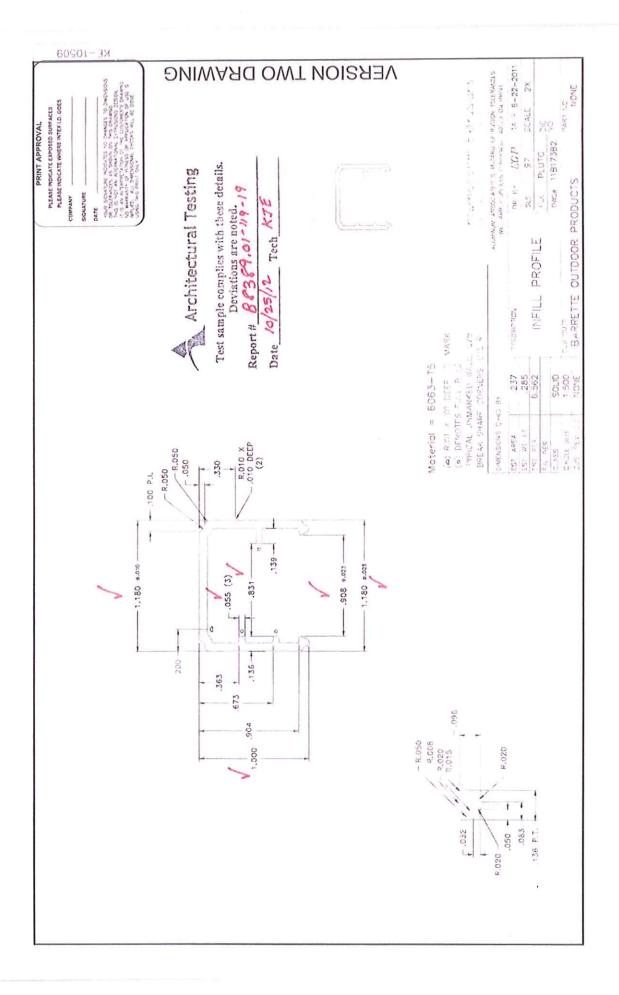
Drawings

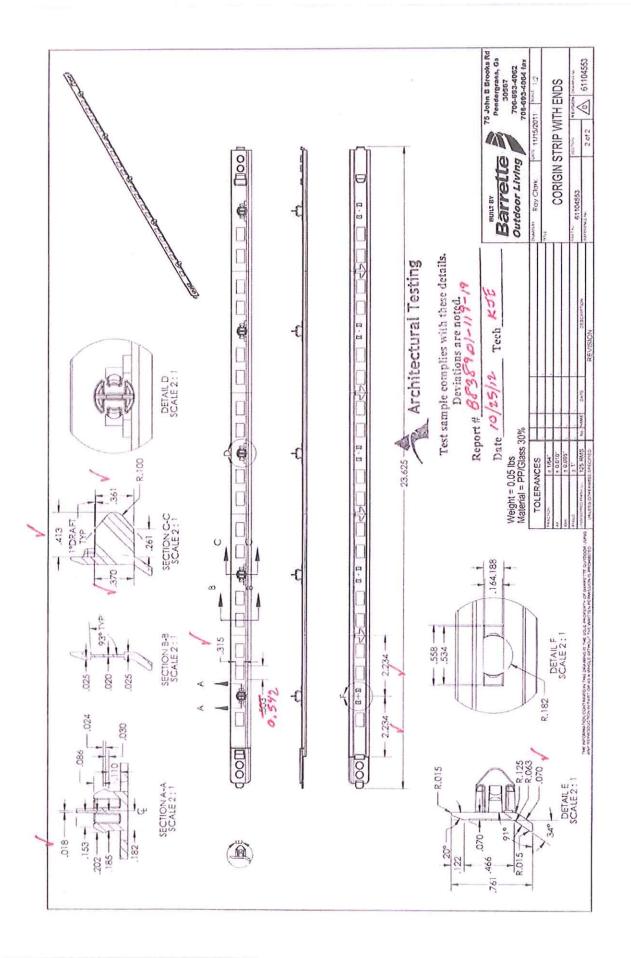


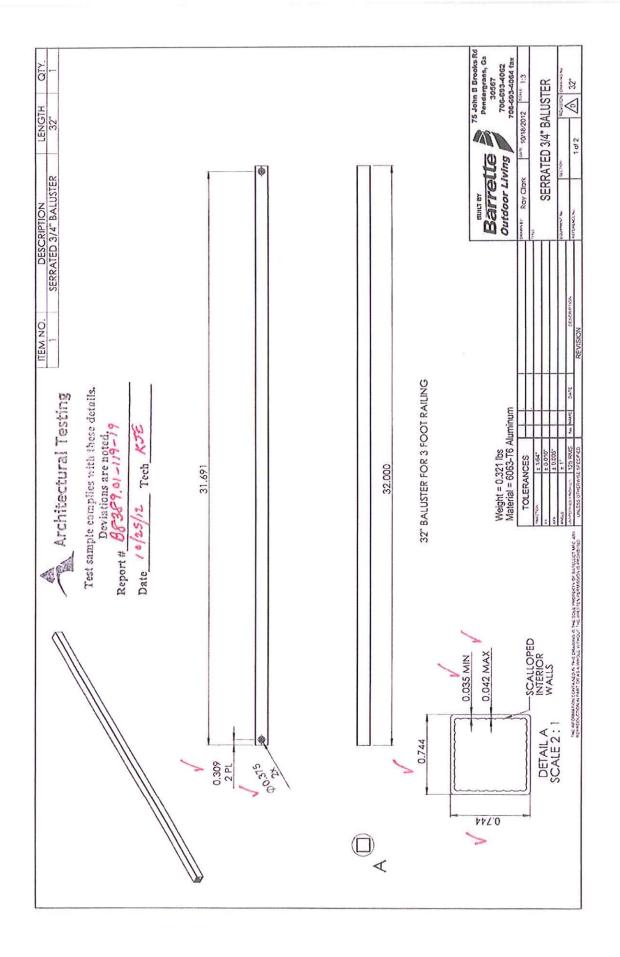


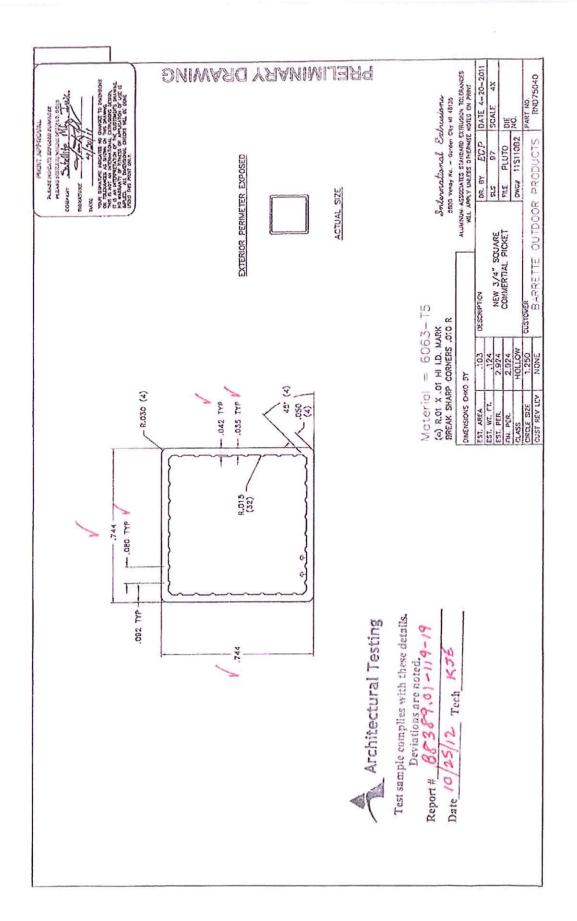


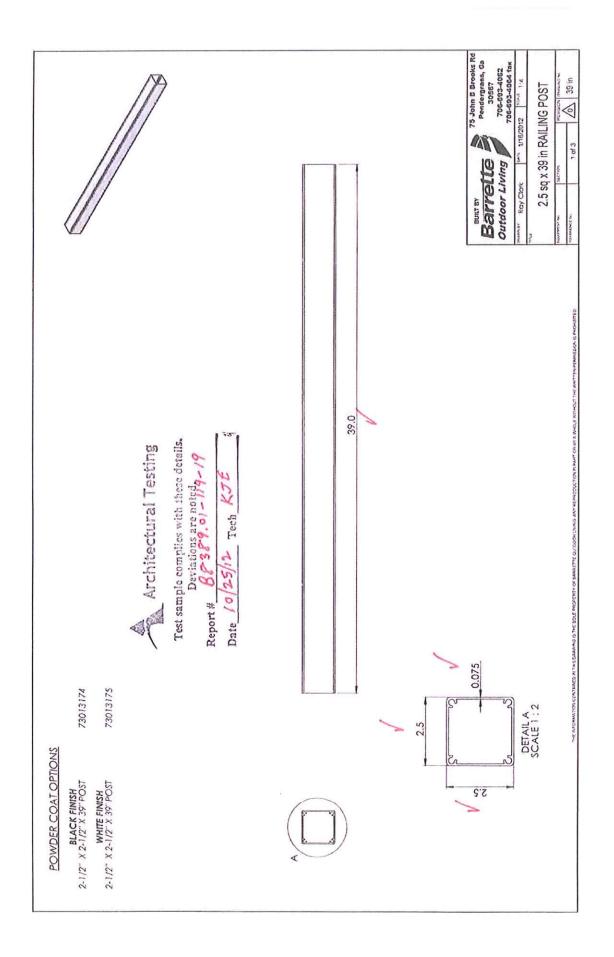


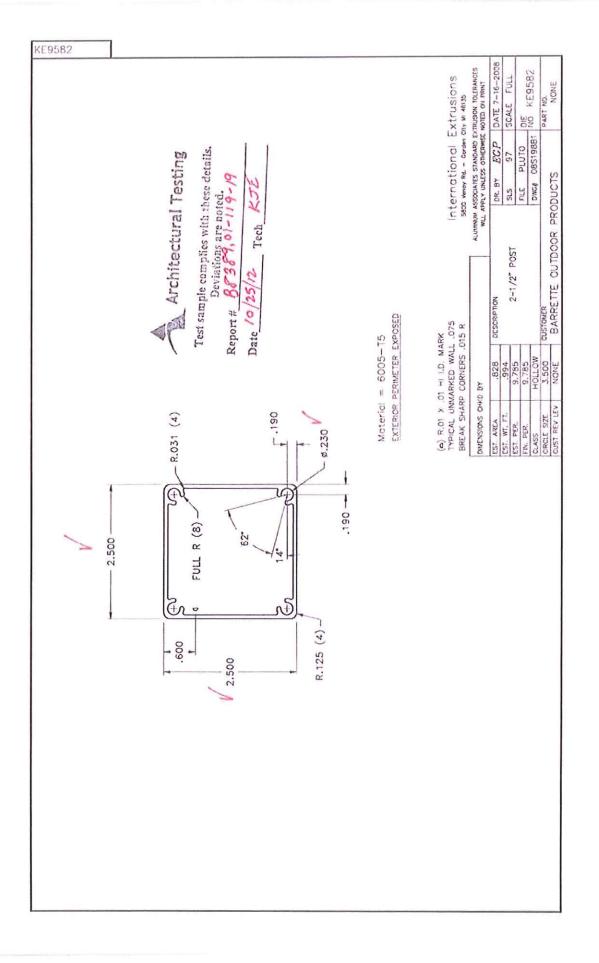


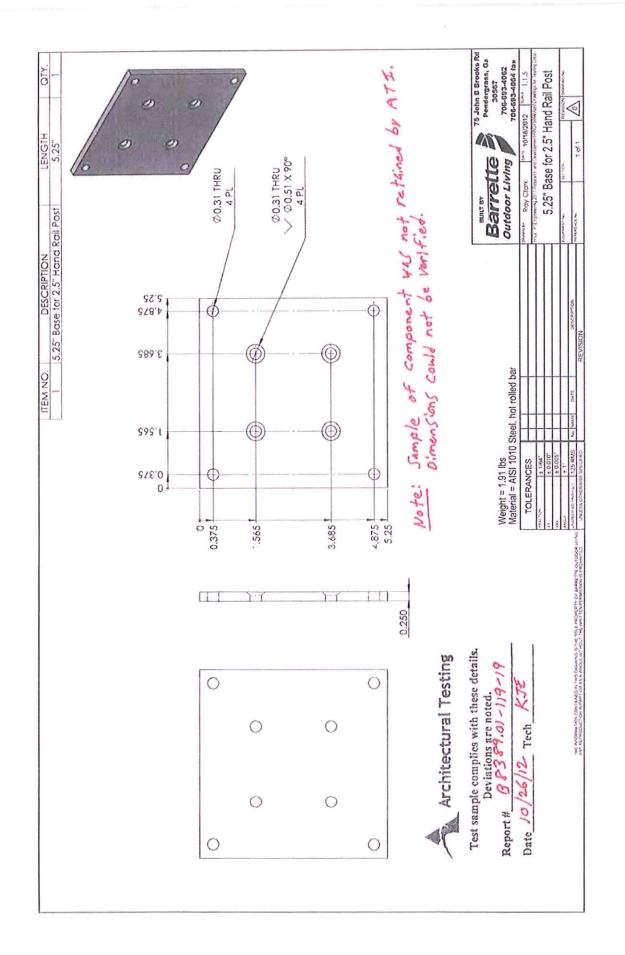


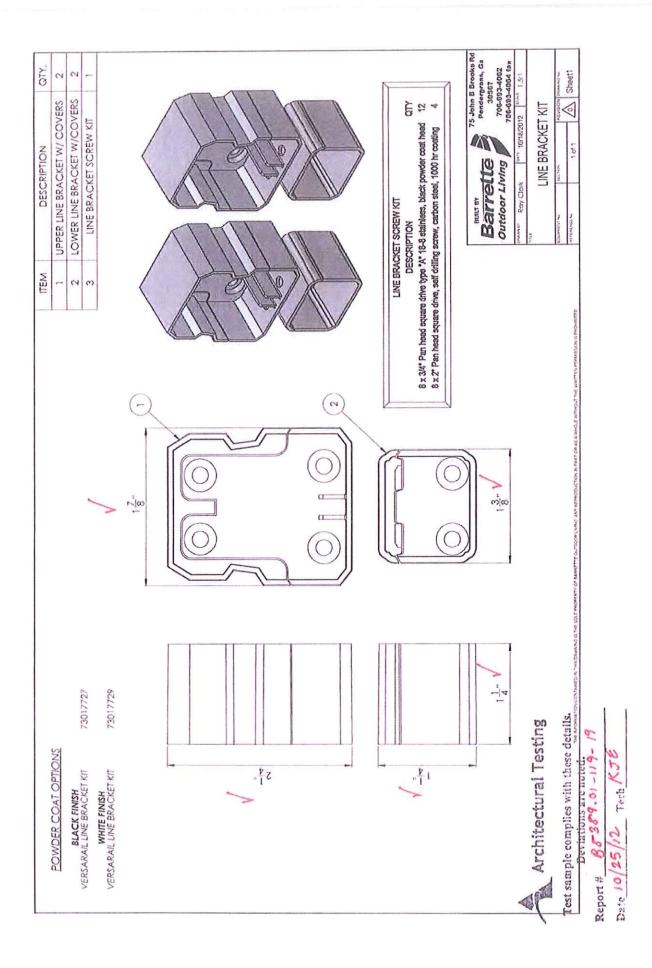


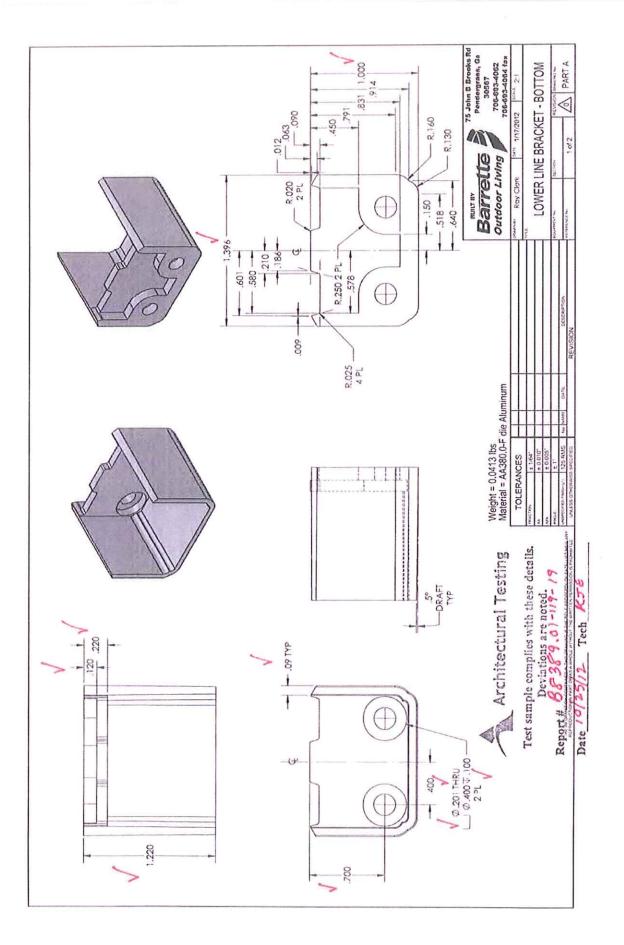


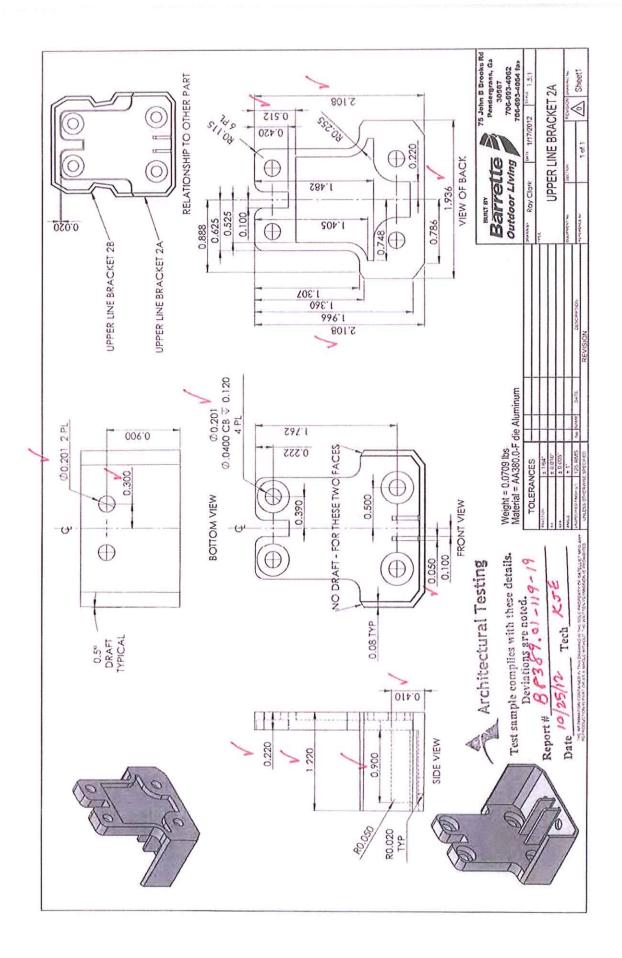














APPENDIX B

Photographs





Photo No. 1 In-Fill Load Test at Center of Two Pickets



Photo No. 2 In-Fill Load Test at Bottom of Two Pickets





Photo No. 3 Concentrated Load Test at Mid-Span of Top Rail



Photo No. 4 Concentrated Load Test at End of Top Rail (Bracket) and Post Tests





Photo No. 5 Post Mount Attached to Mock Wood Deck



Photo No. 6 Top Rail Bracket-to-Rail Connection





Photo No. 7 Bottom Rail Bracket-to-Rail Connection



APPENDIX C

Blocking Instructions

NEW CASTLE / SOMERSET / ELITE ALUMINUM RAIL POST INSTALLATION INSTRUCTIONS

*2 person installation recommended

It is the responsibility of the installer to meet and/or exceed all code and safety requirements, and to obtain all required building permits. The deck and railing installer should determine and implement appopriate installation techniques for each installation. Barrette Outdoor Living and its distributors shall not be held liable for improper or unsafe installations.

Figure 2

What is Included:

- Post
- Mounting Plate -
- Installation Plate
- Base Trim
- ¼" x 2½" Phillips Head Screws

Deck/Wood Surface

- 1.) Attach mounting plate to posts with 1/4" x 21/2" phillips head screws Figure 1.
- 2.) Cut a 2x8 wood spacer block (not included) Figure 2 and attach underneath the deck surface to substructure joists directly under the post location with 3" deck screws (not included). Length of the spacer block should be the distance between the existing deck joists.
- 3.) Cut one 2x8 joist (same length as spacer block cut in Step 3) (not included) Figure 3. Box in the spacer block with this newly cut joist with 3ⁿ deck screws (not included).

Install Posts to Deck

- 4.) Mark holes with a pencil through bottom of installation plate Figure 4 onto deck surface. Drill ¾" holes in all four locations.
- 5.) Push ⁵/₁₆" bolts (not included) through installation plate and attach separate bottom plate Figure 5 (C) from underneath deck surface (pasts can be leveled as needed by using steel washers as shims).
 - Purchase 1/4" bolts with nuts approximately 1" longer than the distance between plates (minimum 37%" long).
- 6.) Install base trim Figure 5 (D) around deck post at deck surface before installing rail.

Concrete Surface

- 1.) Purchase four 5/16" masonry anchors according to local building codes.
- Mark holes through mounting plate onto concrete surface and follow anchor installation instructions.
- 3.) Install base trim Figure 5 (D) around deck post at deck surface before installing rail.

Installing Remainder of Deck Posts

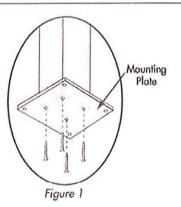
 Measure the length of your rail section and add ³/₄" to measurement for brackets and expansion clearance. This is the distance between possion

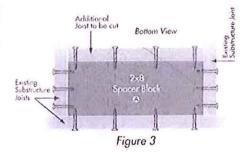
2.) Follow installation instructions from above.

Test sample compiles with these details. Deviations are noted. 19Report # B8389.01 - 119 - 19

Tech

25/12









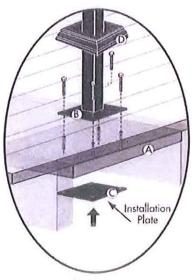
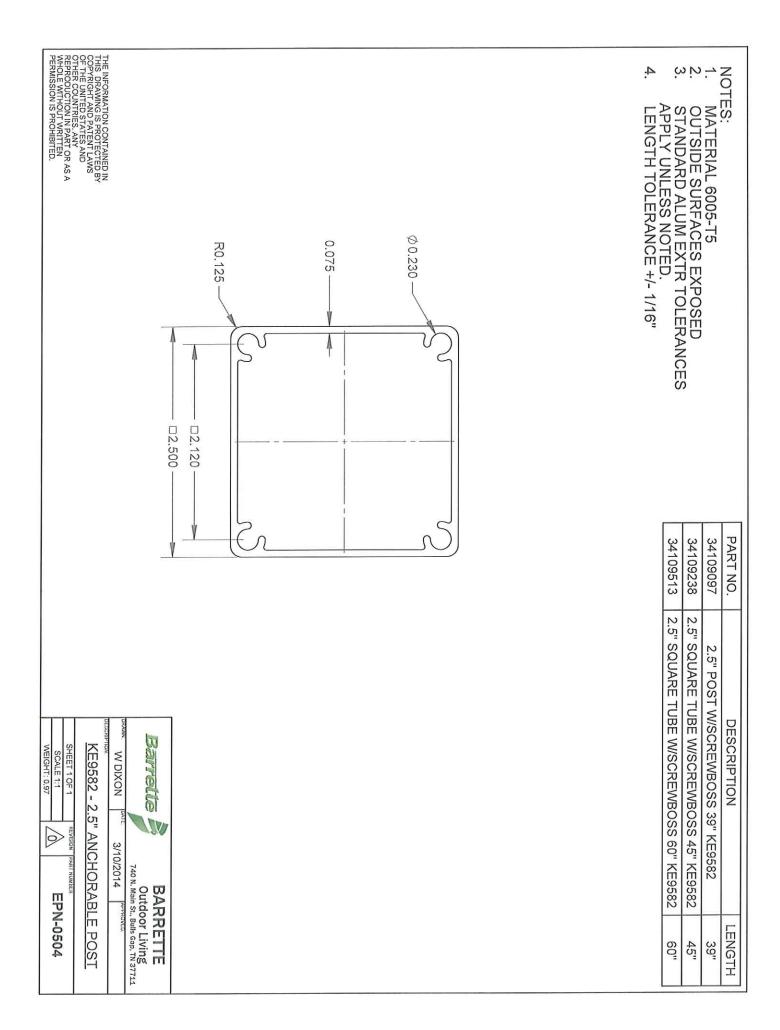
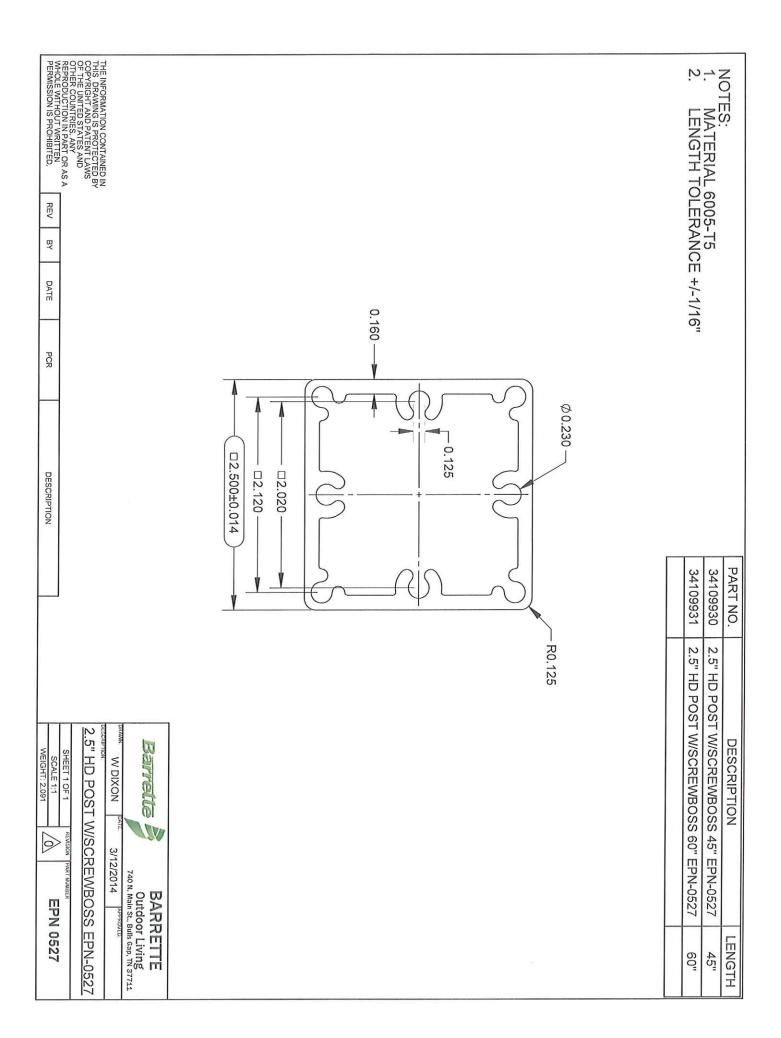


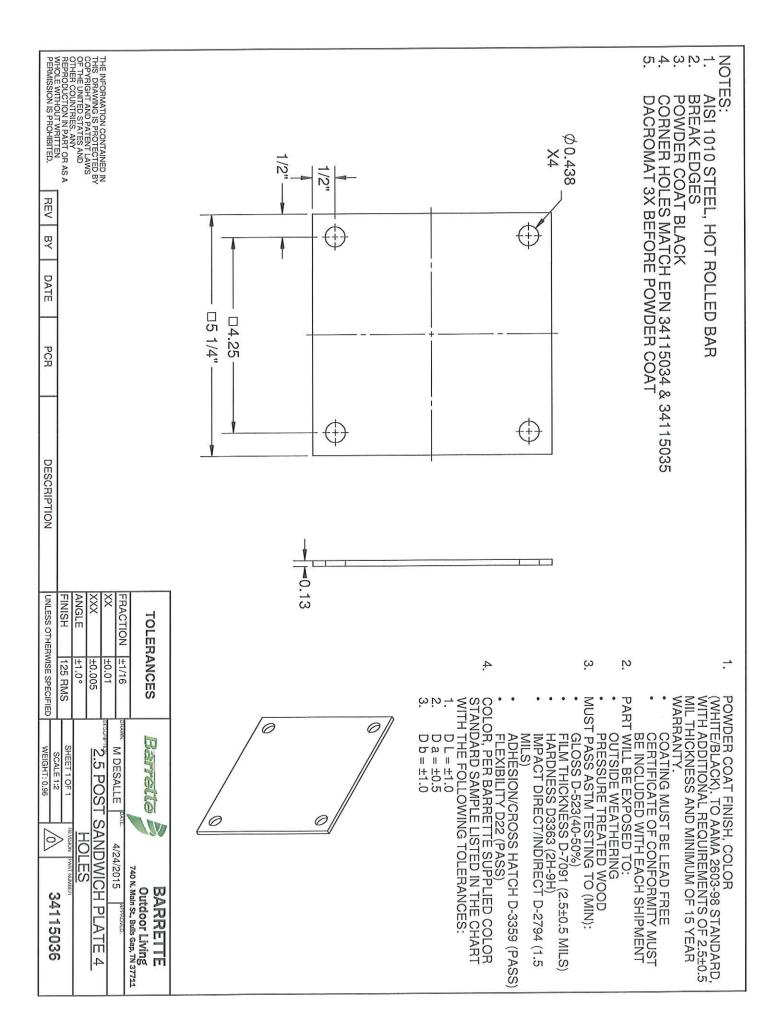
Figure 5

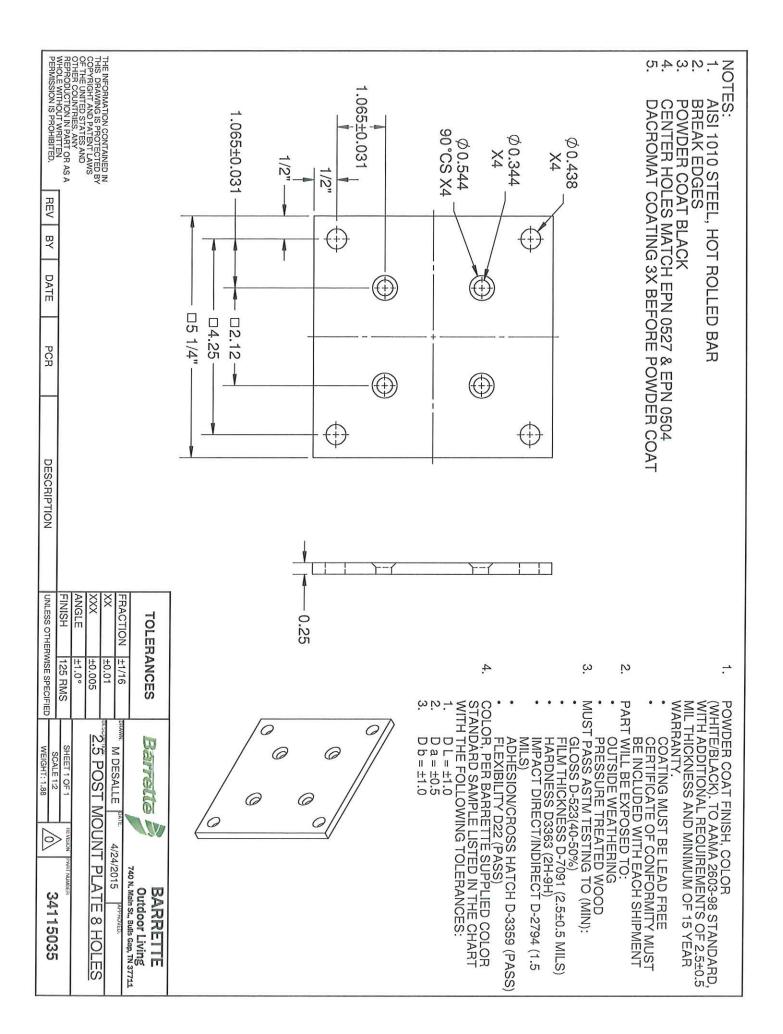
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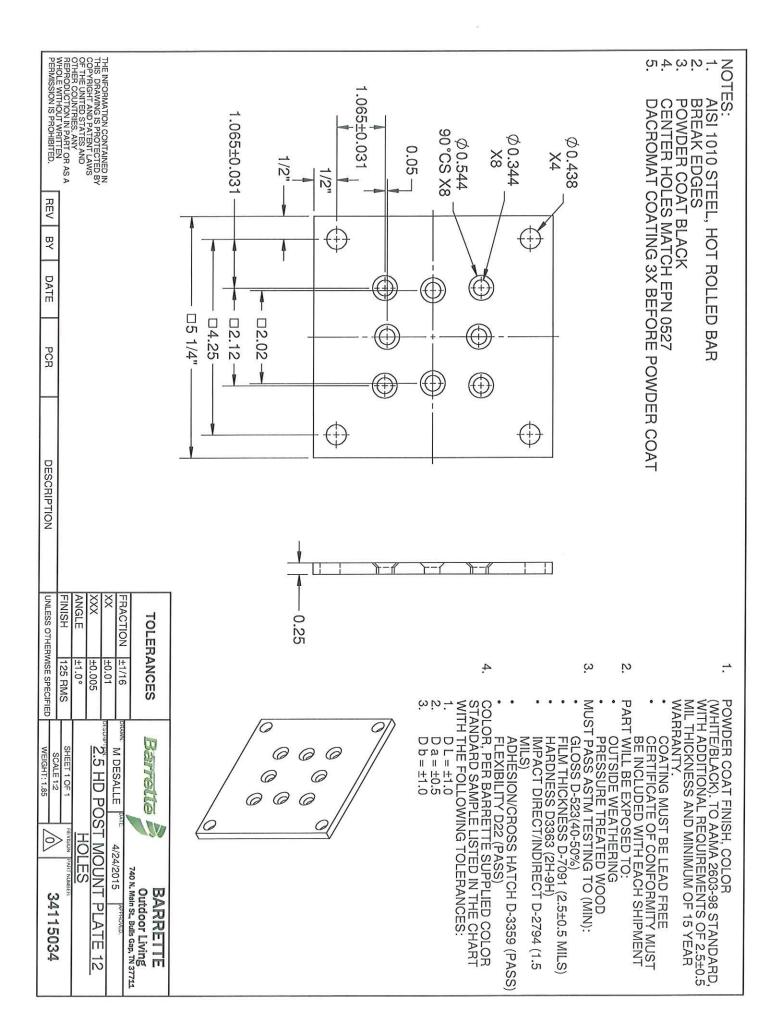
Appendix C

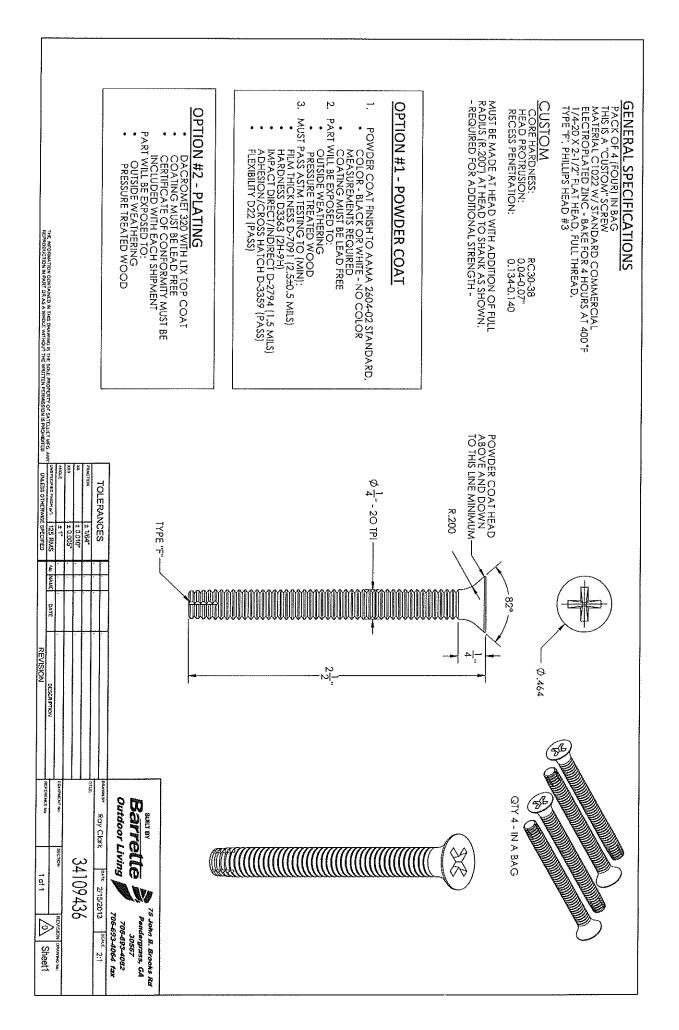


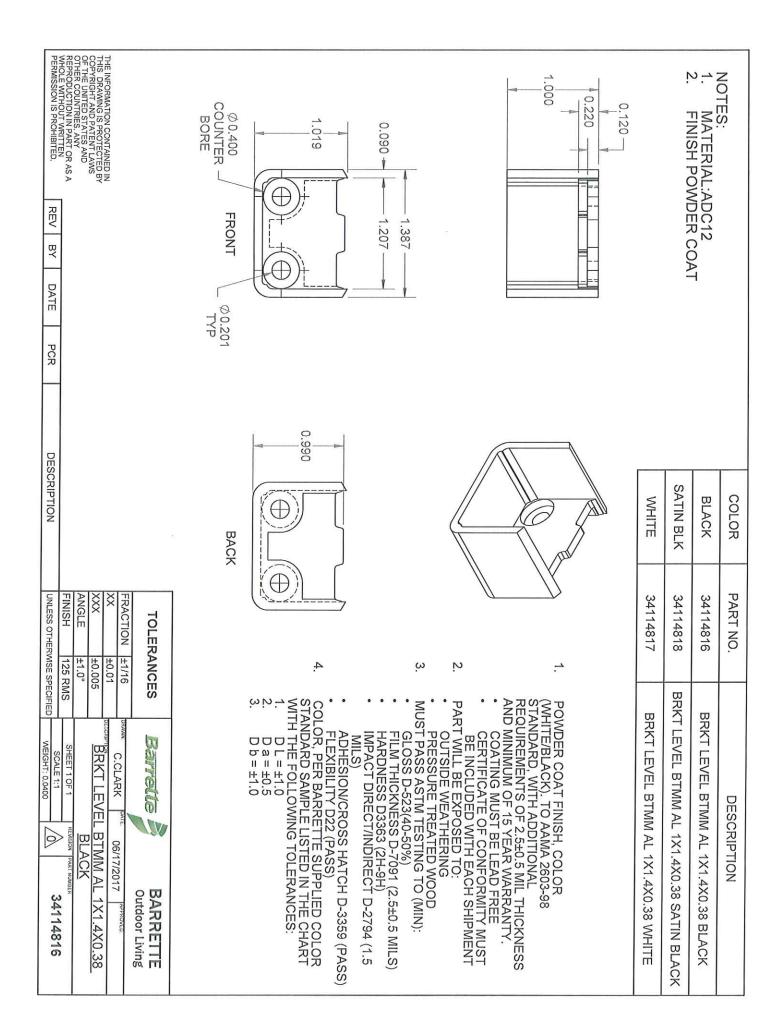


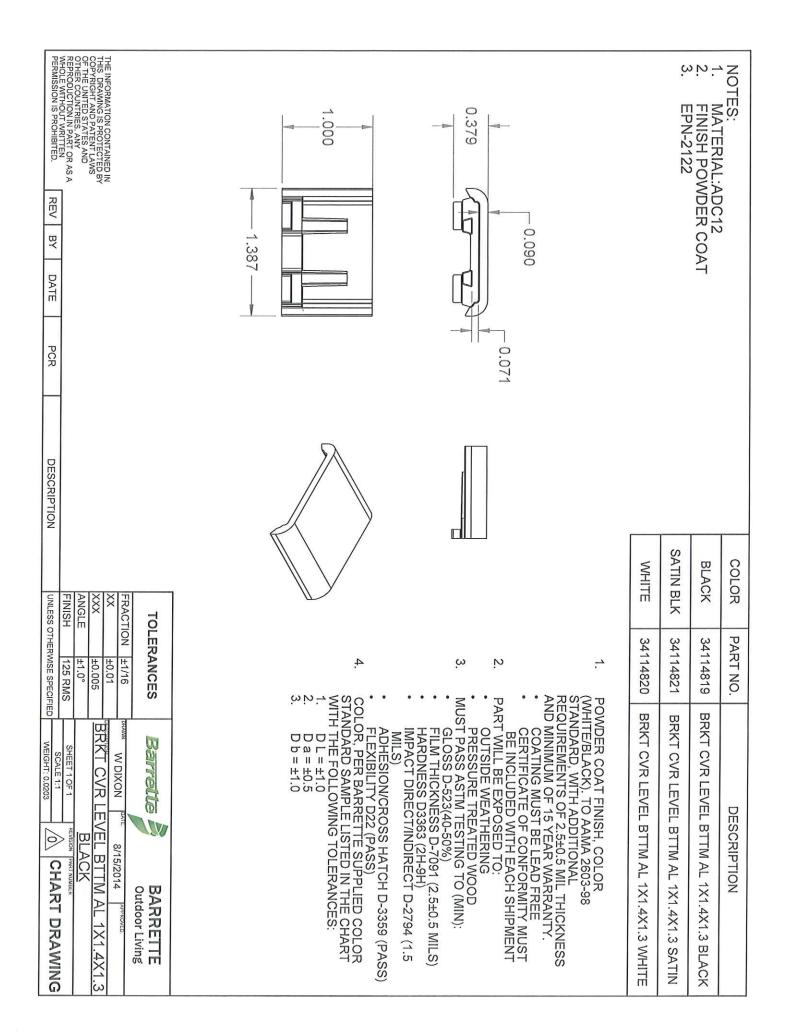








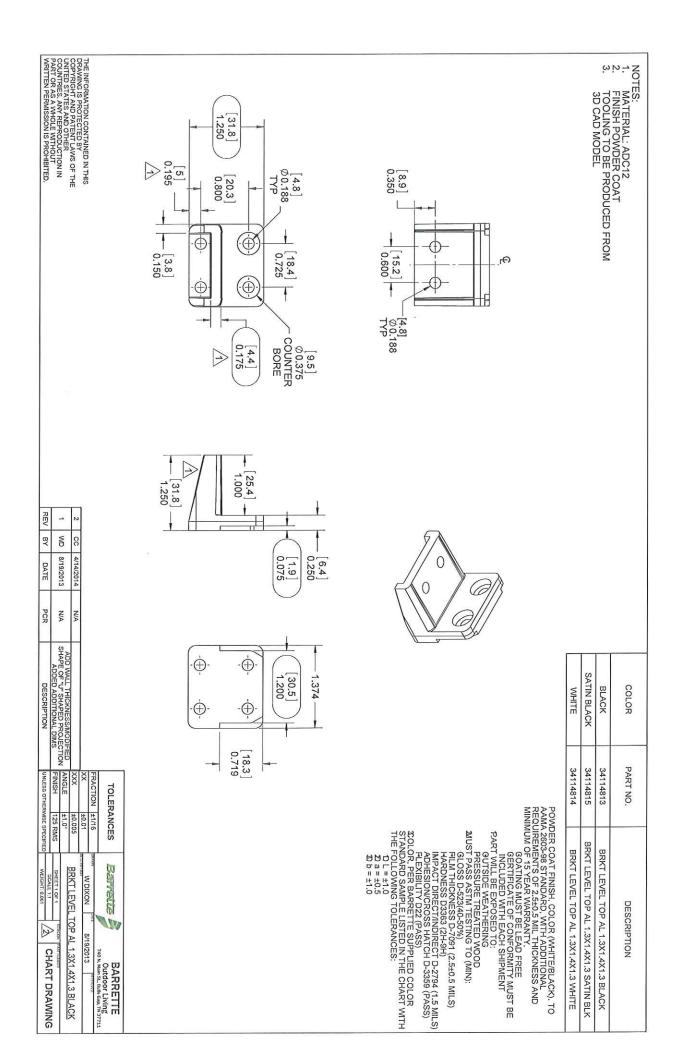


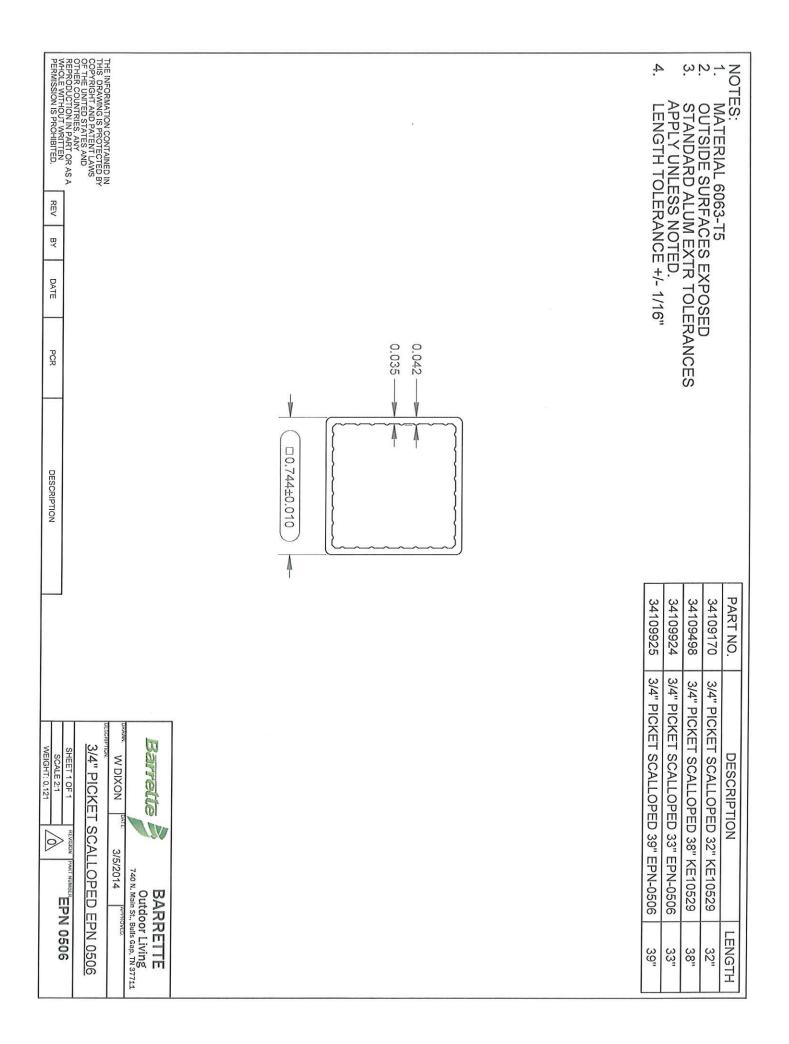


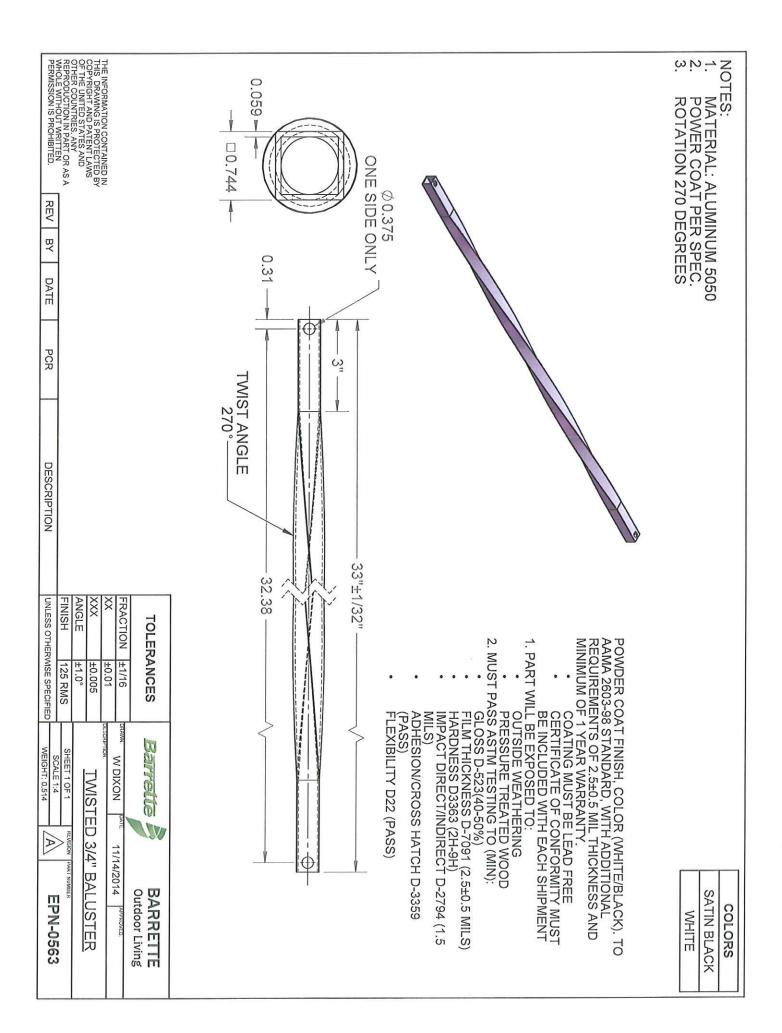
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ВҮ	WED	WED	WED	070±.006	OLER/ 1/16"	OSED
DATE	12/22/2014	2/27/2015	3/18/2015	E-012	ANCES	
PCR	N/A	N/A	N/A			
DESCRIPTION	CHG TOL 1.190 DIM	CHG LENGTH 70.25 TO 69 34107268	ADD TOLERANCES	2.100±.024 .250±.012 - .190+.015 .190035		PART NO. 34107268
3			Barreite B/	FULT TYP		DESCRIPTION TOP RAIL 2.1X1.8X 69 (EPN 0566 AG)
EPN 0566	HANNEL	APPROVED.	BARRETTE Outdoor Living			LENGTH 69"

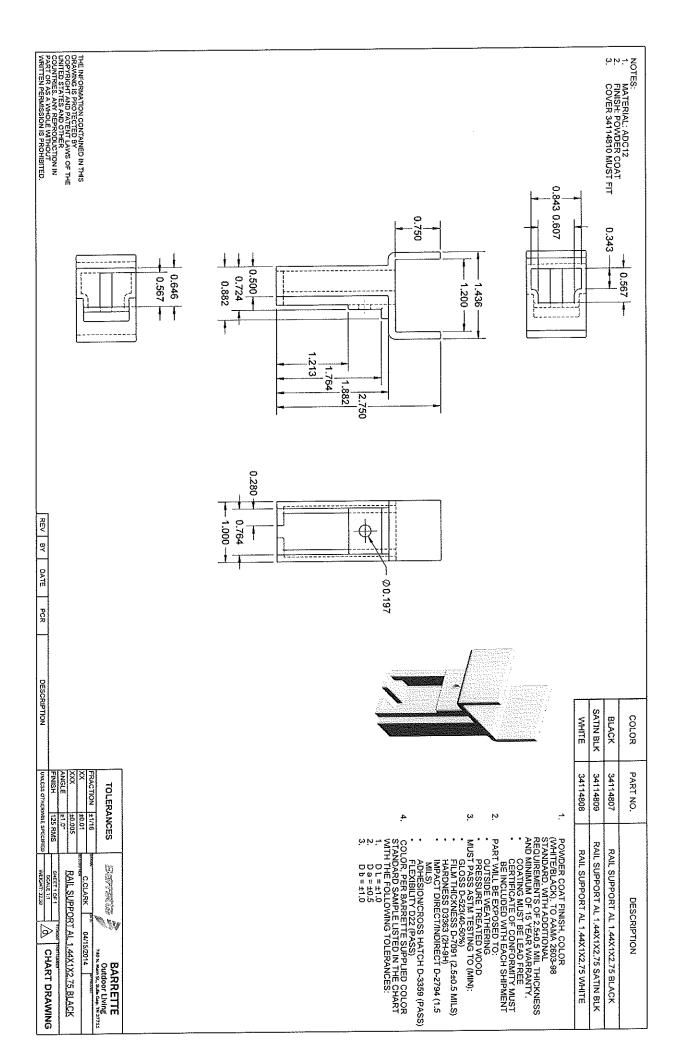
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3 WED 3/18/2015 2 WED 2/27/2015 1 WED 12/22/2014 REV BY DATE	.070±.006 1.000±.012	ES: MATERIAL 6005-T5 OUTSIDE SURFACES EXPOSED SNAPS OVER EPN 0513 STANDARD ALUM EXTR TOLERANCES APPLY UNLESS NOTED. LENGTH TOLERANCE +/- 1/16"
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	FULTYP	DESCRIPTION TOP RAIL 2.05X2.1X 69 (EPN 0565
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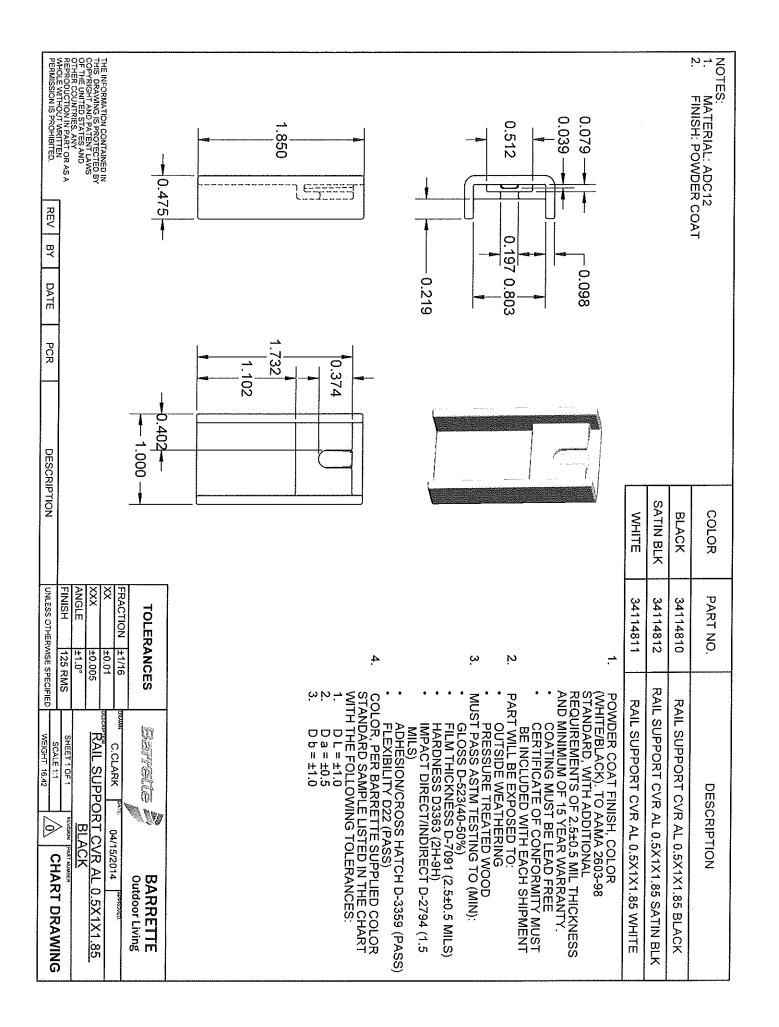
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ВY	۸D		ERAN			RFAC	05-T5	
DATE	3/5/2015		ICE +/- 1,)TED.		ES EXPO	•	
PCR	N/A		'16"		EPN 0507)SED		
DESCRIPTION	ADD 280" LENGTH 34107334	1.180±0.008 AT BASE		Ŭ	SNAPS INTO EXTRUSIONS EPN 0507, 0518, 0519			
		t I	34107334	34109923	34109922	34109921	34109920	PART NO.
	T REVISION PART NUMBER	BARRI Outdoor T-40 N Main St, Bull MUXON TO 1228/2014 To 1000 AVALON CHANNEL	CHANNEL 280" EPN 0513	AVALON CHANNEL 95.5" EPN-0513	AVALON CHANNEL 71.5" EPN-0513	VERSA RAIL CHANNEL 70.25" EPN-0513	VERSA RAIL CHANNEL 93" EPN-0513	DESCRIPTION
EPN 0513		BARRETTE Outdoor Living 12014	280"	95.5"	71.5"	70.25"	93"	LENGTH

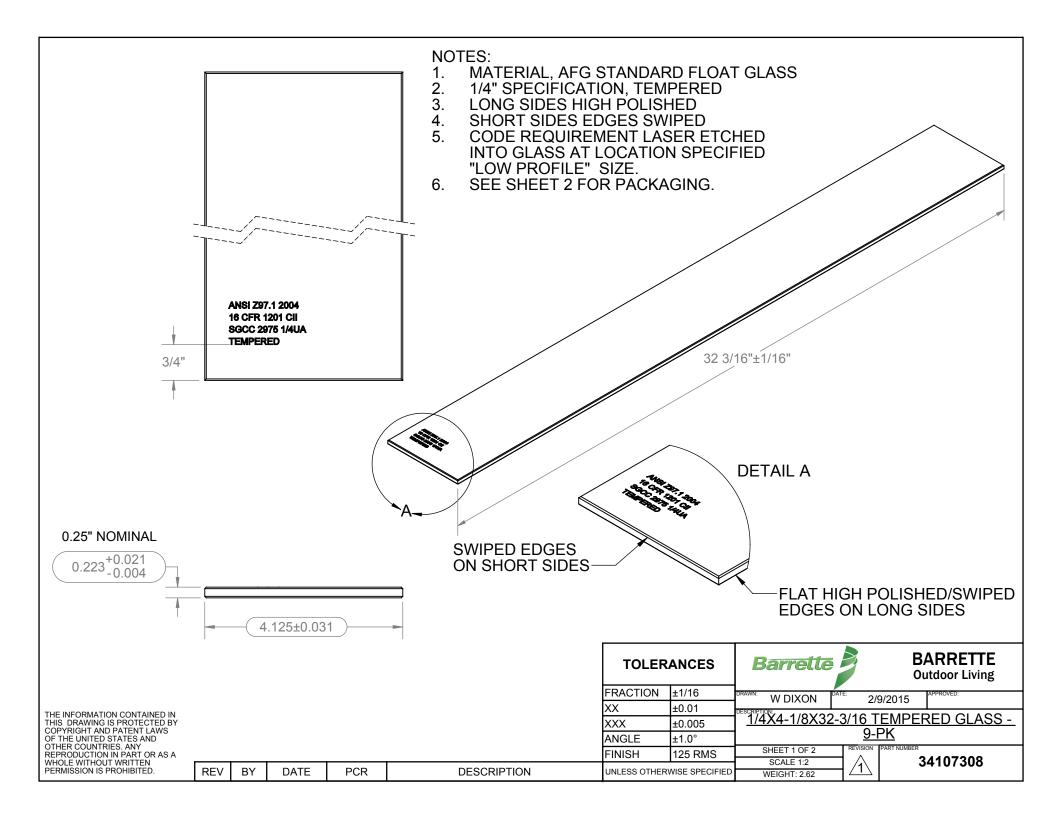


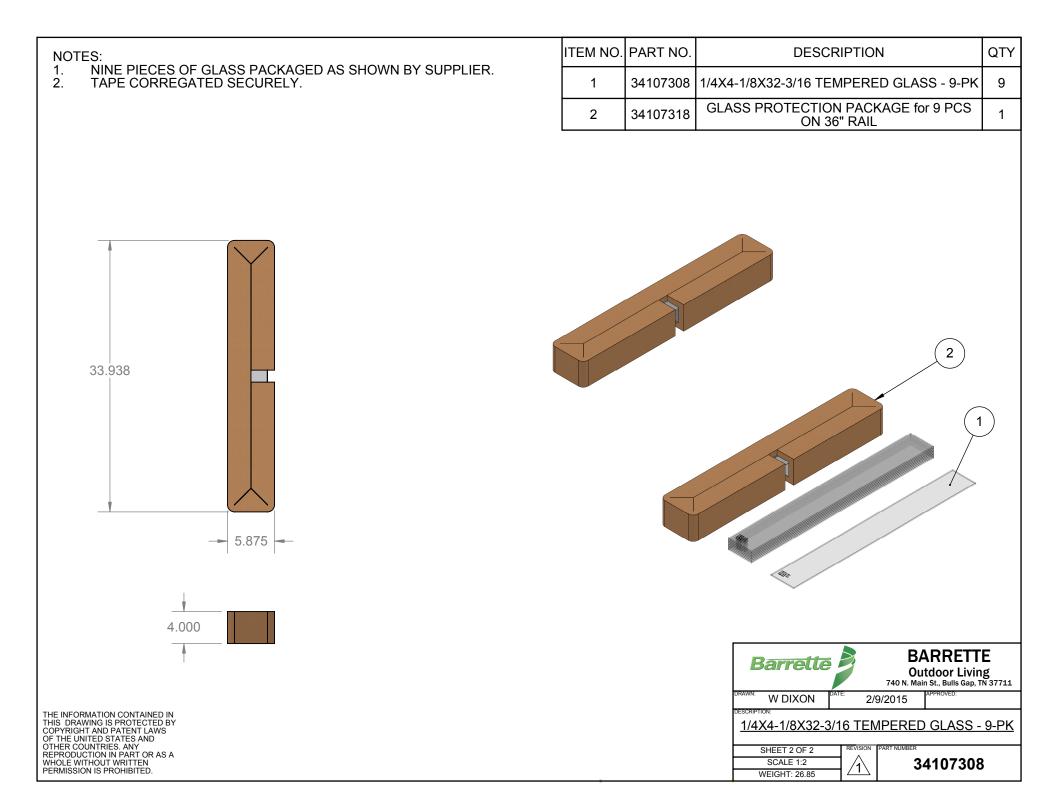








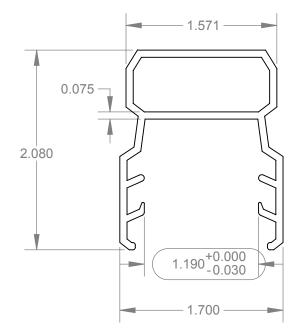




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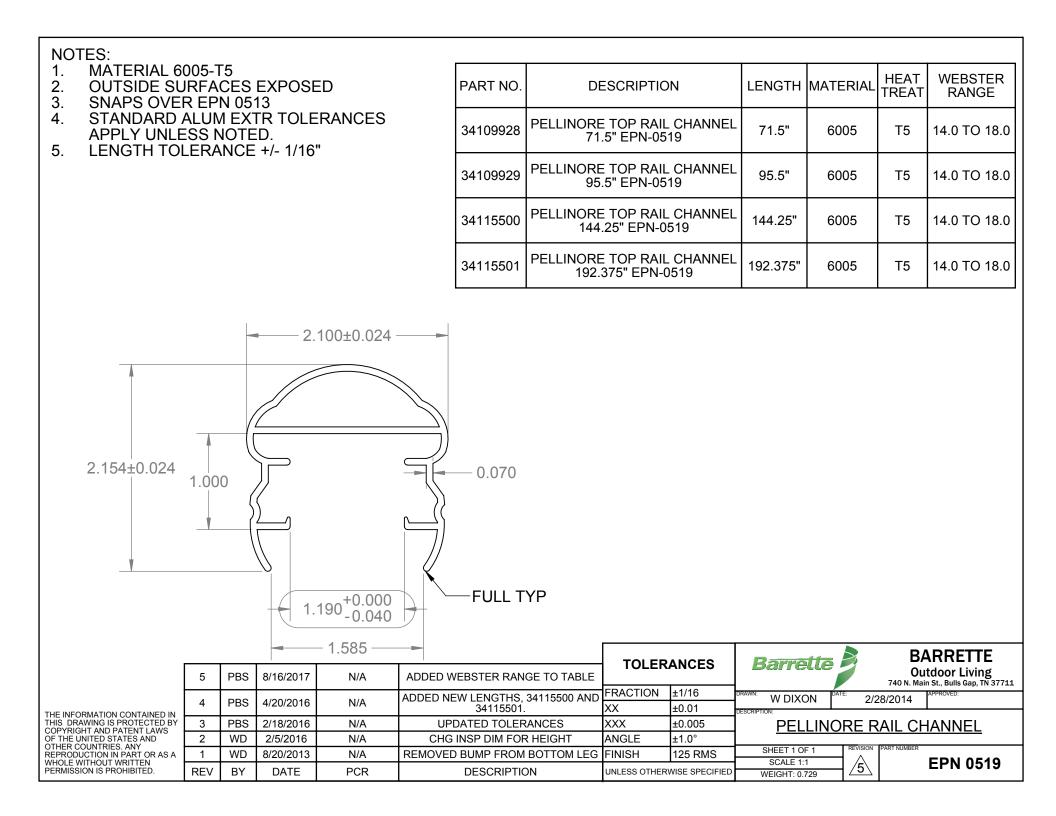
- 1. MATERIAL 6005-T5
- 2. OUTSIDE SURFACES EXPOSED
- 3. SNAPS OVER EPN 0513
- 4. STANDARD ALUM EXTR TOLERANCES APPLY UNLESS NOTED.
- 5. LENGTH TOLERANCE +/- 1/16"

PART NO.	DESCRIPTION	LENGTH
34109496	VERSA RAIL TOP RAIL 70.25" EPN-0507	70.25"
34109194	VERSA RAIL TOP RAIL 93" EPN-0507	93"
34109918	TRISTAN TOP RAIL 71.5" EPN-0507	71.5"
34109919	TRISTAN TOP RAIL 95.5" EPN-0507	95.5"



							Barrette	BARRETTE Outdoor Living 740 N. Main St., Bulls Gap, TN 37711
							W DIXON	ATE: 2/28/2014 APPROVED:
THE INFORMATION CONTAINED IN THIS DRAWING IS PROTECTED BY COPYRIGHT AND PATENT LAWS OF THE UNITED STATES AND								SARAIL PROFILE
OTHER COUNTRIES. ANY REPRODUCTION IN PART OR AS A WHOLE WITHOUT WRITTEN						1	SHEET 1 OF 1 SCALE 1:1	
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NC 1. 2. 3.	OTES: MATERIAL 6 OUTSIDE SU SNAPS OVE	JRFA	CES		ED	PART NO.		RIPTION CHANNEL 71.5" EPN-	LENGTH	MATERIAL	HEAT TREAT	WEBSTER RANGE
4.	STANDARD	ALUN	1 EX1	FR TOLE	ERANCES	34109926	05	518	/1.5	6005	T5	14.0 TO 18.0
5.	APPLY UNLE LENGTH TO				;"	34109927		CHANNEL 95.5" EPN- 518	95.5"	6005	T5	14.0 TO 18.0
			2.1	30	R1.050		2.100	-FULL TYP	Barra	ette 🌶		RETTE
								FRACTION ±1/16	DRAWN: W DIX	DATE	740 N. Main St 8/2014	DOR Living , Bulls Gap, TN 37711 ROVED:
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REPRO	COUNTRIES. ANY DUCTION IN PART OR AS A WITHOUT WRITTEN	1	PBS	8/16/2017	N/A		STER RANGE TO TABLE	FINISH 125 RMS	SHEET 1 C SCALE 1			PN 0518
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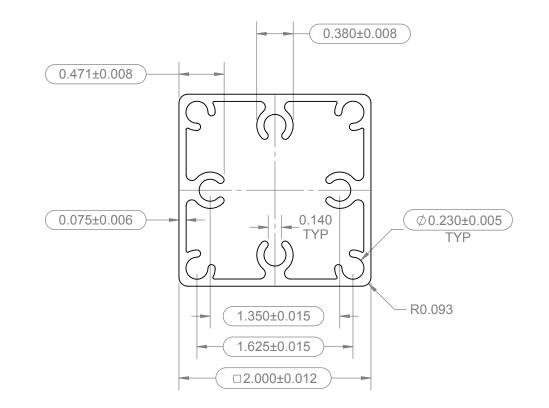


NOTES: 1. MATERIAL 2. OUTSIDE S 3. STANDARD APPLY UNL 4. LENGTH TO	URFA ALU ESS	ACES M EX NOTI	TR TOL ED.	ERANCES	CATED		DESCRIPTIONLENGTHOBERON TOP RAIL EPN-0743 71.571.5"OBERON TOP RAIL EPN-0743 95.595.5"
3°		0.07		= 0.737 in 3.083 - Q -1.571 - 2.08 1.190 ^{+0.000} 1.190 ^{-0.030}			EXPOSED SURFACES
THE INFORMATION CONTAINED IN THIS DRAWING IS PROTECTED BY COPYRIGHT AND PATENT LAWS OF THE UNITED STATES AND OTHER COUNTRIES, ANY							Barrette Barrette Outdoor Living 740 N. Main St., Bulls Gap, TN 37711 DRAWN: W DIXON PATE: 8/17/2017 APPROVED: DESCRIPTION: OBERON TOP RAIL SHEET 1 OF 1 REVISION:
REPRODUCTION IN PART OR AS A WHOLE WITHOUT WRITTEN PERMISSION IS PROHIBITED.	REV	BY	DATE	PCR	l	DESCRIPTION	SHEET 1 OF 1 SCALE 1:1 WEIGHT: 0.863

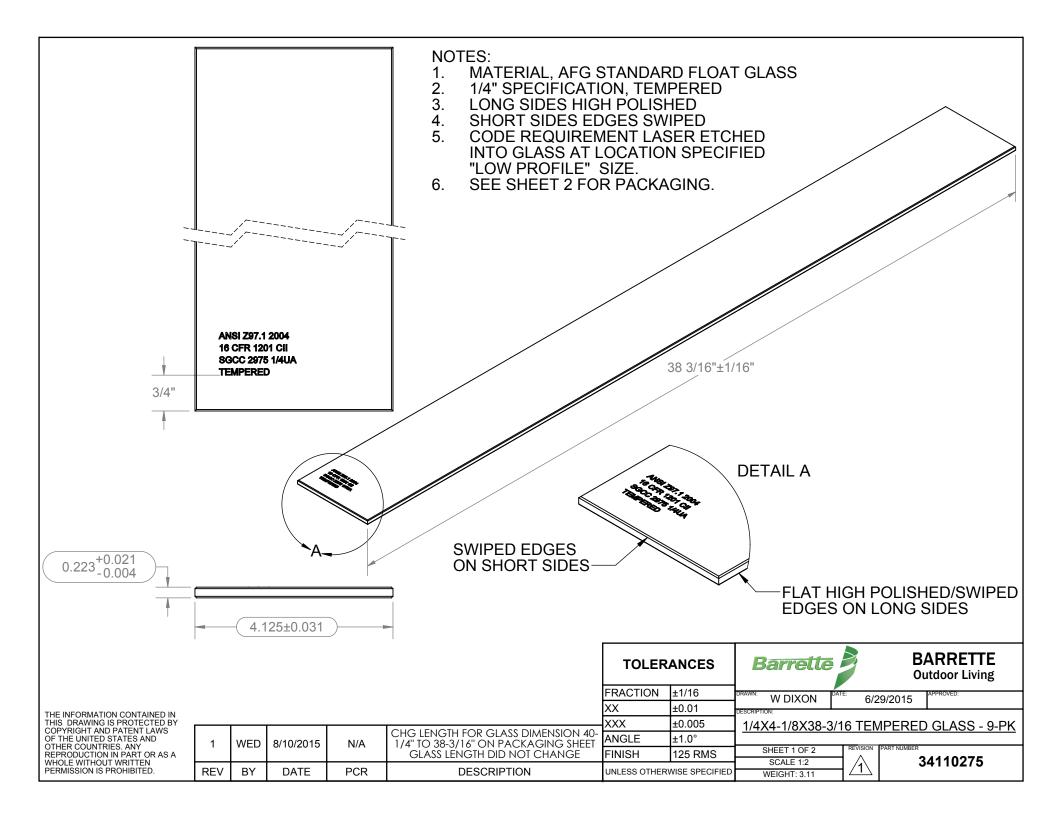
NOTES:

- MATERIAL: 6005-T5 / 6005A-T61 1.
- 2. 3. ALUMINUM ASSOCIATION STANDARD TOLERANCES APPLY.
- LENGTH TOLERANCE +/- 1/16" OUTSIDE SURFACES EXPOSED
- 4.

PART NO.	DESCRIPTION	LENGTH
34107348	EPN 0541 2X2 POST 33"	33"
34107349	EPN 0541 2X2 POST 39"	39"
34107350	EPN 0541 2X2 POST 45"	45"
34107351	EPN 0541 2X2 POST 51"	51"



						TOLER	ANCES	Barrette		BARRETTE Outdoor Living
						FRACTION	±1/16		E: 1/9	9/2015
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THIS DRAWING IS PROTECTED BY COPYRIGHT AND PATENT LAWS	2	PBS	9/22/2015	N/A	UPDATED LENGTH DIMENSIONS	XXX	±0.005		<u>2X2 F</u>	<u>2051</u>
OF THE UNITED STATES AND			2/9/2015		ADD TOLEANCES AND 6005A-T61 ALLOY	ANGLE ±1.0°	±1.0°			
OTHER COUNTRIES. ANY REPRODUCTION IN PART OR AS A	1	WED		N/A		FINISH	125 RMS	SHEET 1 OF 1	REVISION	
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PERMISSION IS PROHIBITED.	REV	BY	Y DATE			UNLESS OTHERWISE SPECIFIE		WEIGHT: 1.137		



NOTES:

- NINE PIECES OF GLASS PACKAGED AS SHOWN BY SUPPLIER. TAPE CORREGATED SECURELY. 1.
- 2.

ITEM NO	. PART NO.	DESCRIPTION	QTY
1	34110275	1/4X4-1/8X38-3/16 TEMPERED GLASS - 9-PK	9
2	EPN-5420	GLASS PROTECTION PACKAGE FOR 9 PCS ON 42" RAIL	1
		The second secon	g N 37711
		DRAWN W DIXON DATE 6/29/2015 DESCRIPTION: <u>1/4X4-1/8X38-3/16 TEMPERED GLASS -</u>	<u>9-PK</u>
*		SHEET 2 OF 2 SCALE 1:2 WEIGHT: 31.82	;

